

#### **Revision of the South African Pavement Design Method Phase 3**

Road Pavements Forum 13 May 2015 Project SAPDM/D-3: Stabilised Material H Theyse



## R35 Bethal - Objectives

- Assess properties of BSM foam, BSM emulsion and cement treated sections
  - Initial properties
  - Medium-term changes in properties
  - Compare the performance of BSM foam, BSM emulsified and cement stabilised sections under similar traffic and environmental conditions
  - Compare the performance of thin asphalt (AC) and Cape seal (S4) on the above base types
  - Assess the accuracy of the design models P available (2012) for stabilised material

## **R35 Bethal - Status in November 2012**

- Planning document completed
- Pavement and mix design completed
- Construction
  - Southbound base construction
    - Started on 11 April 2012
    - Completed on 7 May 2012
  - Northbound base construction
    - Started on 1 August 2012
    - Completed on 6 September 2012









#### Section identification

Material type	Material symbol	Lime [%]	Cement [%]	Residual binder [%]		
<b>Cement stabilised</b>	C3	1 %	2 %	NA		
	ETB1	NA	1 %	2.4 %		
<b>BSM</b> emulsion	ETB2	NA	2 %	2.4 %		
	ETB3	NA	1 %	0.9 %		
BSM foam	FTB1	NA	1 %	2.4 %		
DOMINAL	FTB2	NA	2 %	2.4 %		





#### Southbound

	Cement con	tent:	2%	2%	1%	1%	2%	2%	1%	1%	2%	2%
	Lime con	tent:	1%	1%	0%	0%	0%	0%	0%	0%	0%	0%
	Bitumen con	tent:	0%	0%	2.4%	2.4%	2.4%	2.4%	2.4%	2.4%	2.4%	2.4%
1	2		3	4	5	6	7	8	9	10	11	12
425	575		350	350	350	350	350	350	350	350	350	350
G1(1)	G1(2)											
200 C3 Sub(1)	200 C3 Sub(2	2)	200 C3(1)	200 C3(2)	175 ETB1	200 ETB1	175 ETB2	200 ETB2	175 FTB1	200 FTB1	175 FTB2	200 FTB2
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#### Northbound

		Cement content:	2%	1%	1%	1%	2%	2%	1%	1%	2%	2%
		Lime content:	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%
	E	Bitumen content:	0%	0.9%	2.4%	2.4%	2.4%	2.4%	2.4%	2.4%	2.4%	2.4%
1	2	3	4	5	6	7	8	9	10	11	12	13
333	333	334	350	350	350	350	350	350	350	350	350	350
G1(1)	G1(2)	G1(3)										
200 C3 Sub(1)	200 C3 Sub(2)	200 C3 Sub(3)	200 C3(1)	200 ETB3	175 ETB1	200 ETB1	175 ETB2	200 ETB2	175 FTB1	200 FTB1	175 FTB2	200 FTB2
333	333	334	350	350	350	350	350	350	350	350	350	350
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#### **Curing of stabilised sections**

Short-term properties of stabilised material

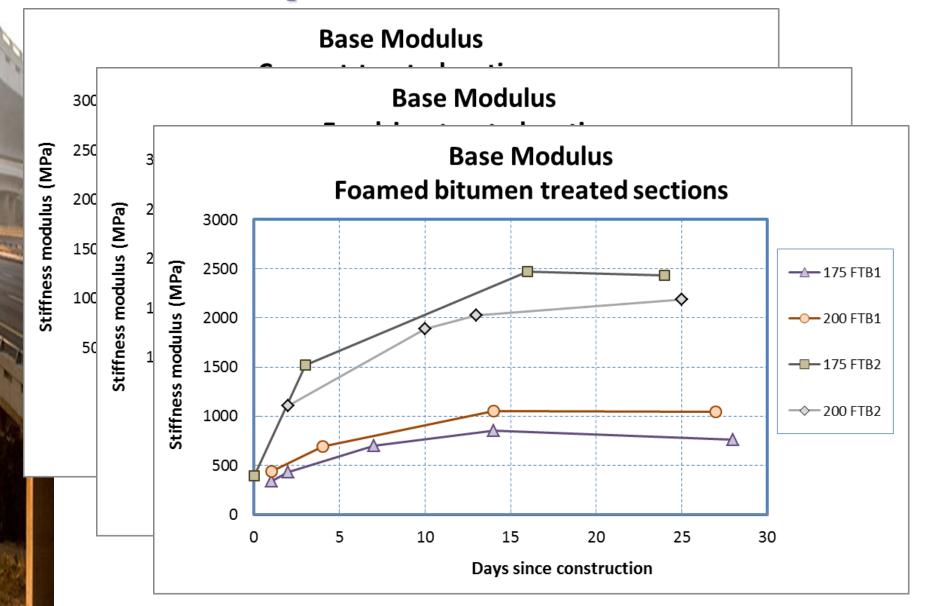


# READS AGENCY MONITORING of curing period

Days since construction		Tests	Responsibility
1 day	1)	Visual condition	Site supervision
	2)	FWD	SRT
	3)	LWD	CSIR/SSI
	4)	3 x cores for UCS, ITS and moisture content	Site supervision
7 days	1)	FWD	SRT
	2)	LWD	CSIR/SSI
	3)	3 x cores for UCS, ITS and moisture content	Site supervision
14 days	1)	FWD	SRT
	2)	LWD	CSIR/SSI
	3)	3 x cores for UCS, ITS and moisture content	Site supervision
28 days	1)	FWD	SRT
	2)	LWD	SSI
	3)	DCP	PMC
	4)	3 x 150 mm Ø cores for UCS and MC	Site supervision
	5)	3 x 150 mm Ø cores for ITS and MC	
	6)	10 x 150 mm Ø cores for tri-axial tests and MC	
	7)	1 x 500 mm x 500 mm slab for flexural beam tests	



#### Monitoring of curing period: FWD temporal variation on SB





#### **2-Year Performance Assessment**

# Medium-term properties of stabilised material





## **Medium-term monitoring**

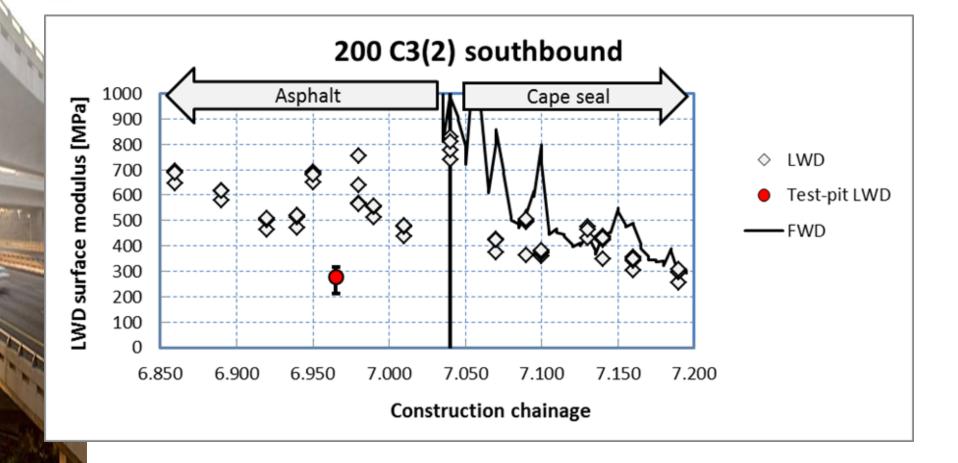
- FWD deflection recorded on all sections up to Aug 2014
- Detail investigation during April 2014
  - Not full 2 year service assess condition after relatively wet summer
  - Sections revisited
    - 1) 200 C3(2) SB
    - 2) 200 ETB2 SB
    - 3) 200 FTB2 SB
    - **4)** 200 FTB1 km 22
    - **5)** 200 FTB1 km 30



# **200 C3(2) SB – Visual Condition**

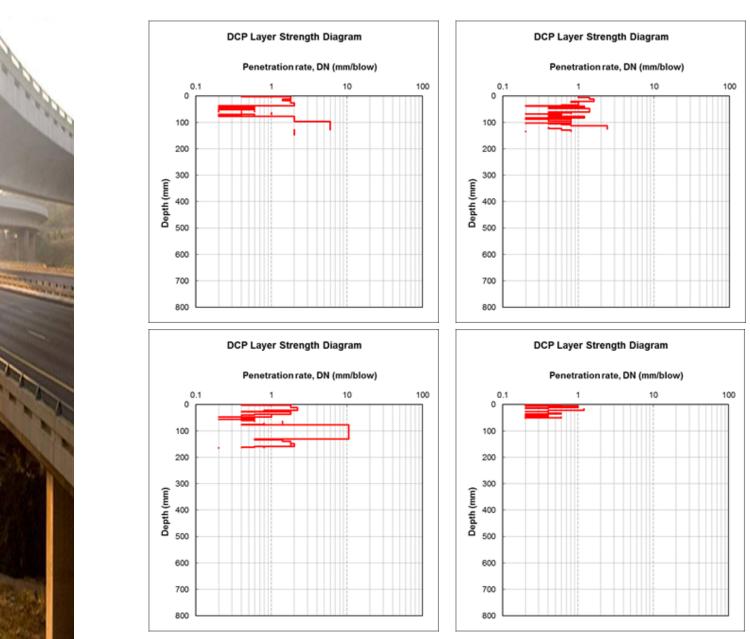


# **EXAMPLE 200 C3(2) SB – Deflection**





#### **EXAMPLE AN HATCOME 200 C3(2) SB – DCP**



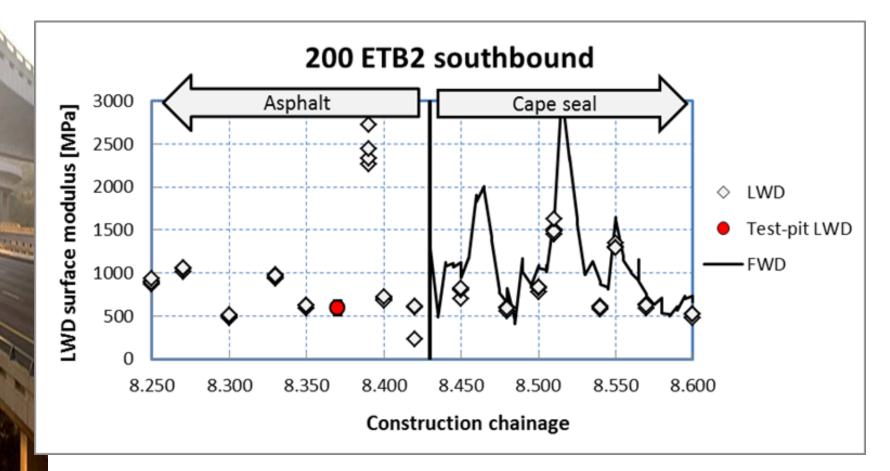


# **EXAMPLE A VISUAL CONDITION**



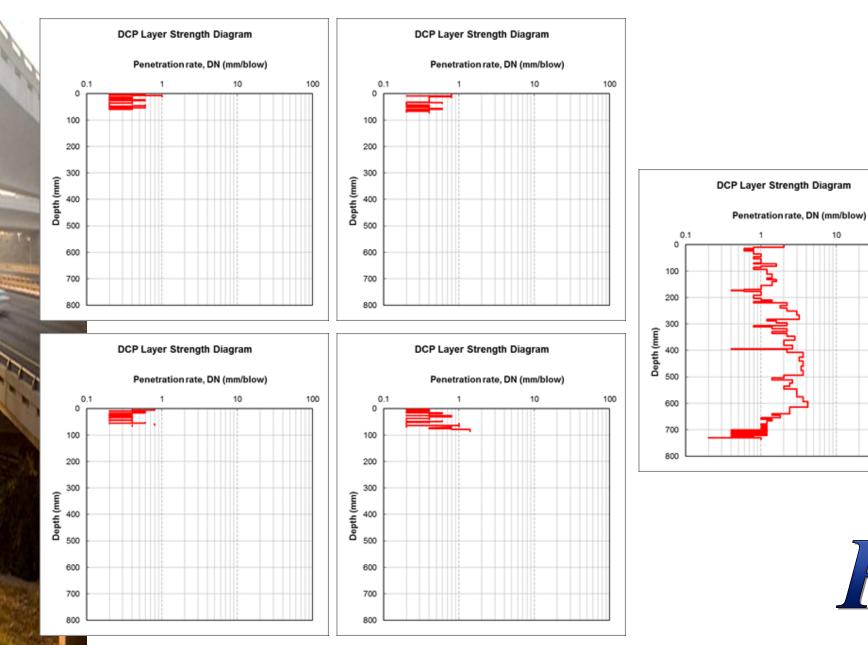
 $P_m^c$ 

# **EXAMPLE A VALUE AND A VALUE A**





# RUADS AGENCY: 200 ETB2 SB – DCP



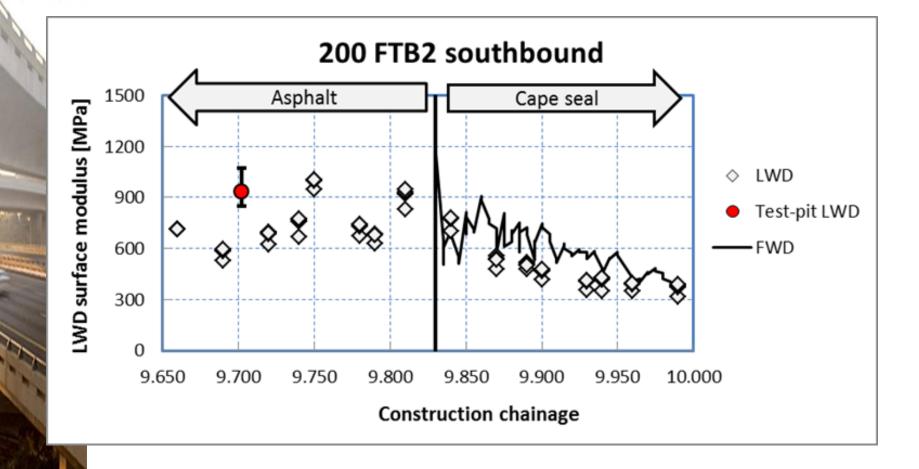
100

# **200 FTB2 SB – Visual Condition**



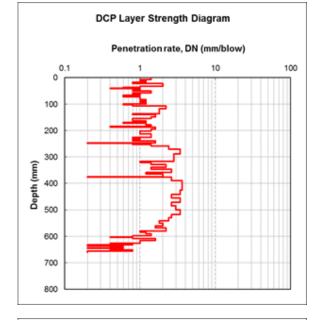


# **EXAMPLE A CONTACT AND A CONTA**

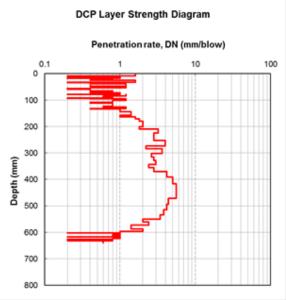


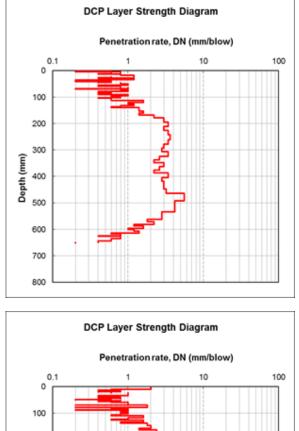


# ROADS AGENCY



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Depth (mm)



## 200 FTB2 km 22 – Visual February 2014



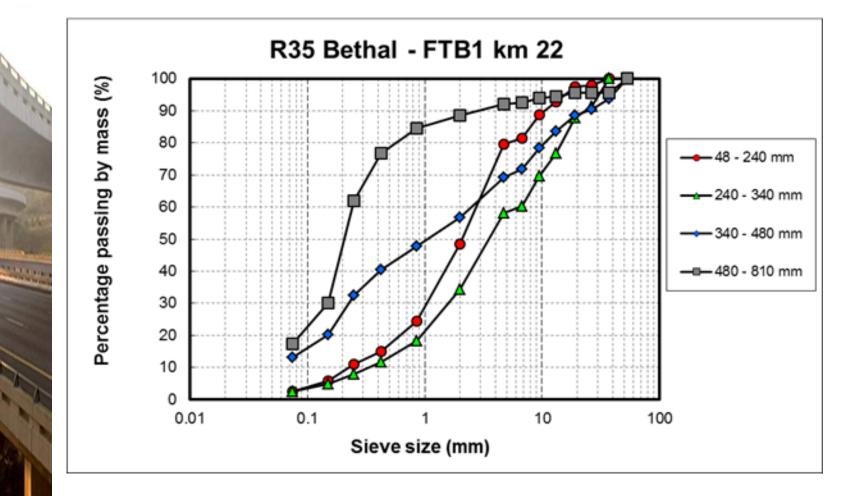






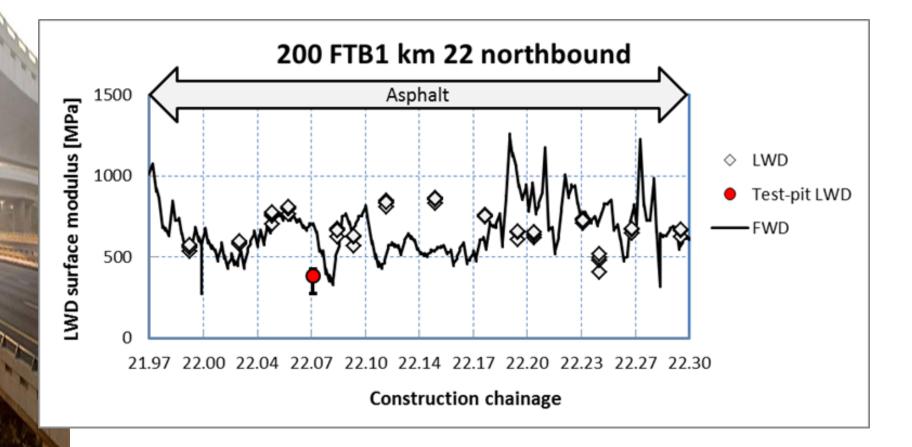


# 200 FTB2 km 22 NB – Grading



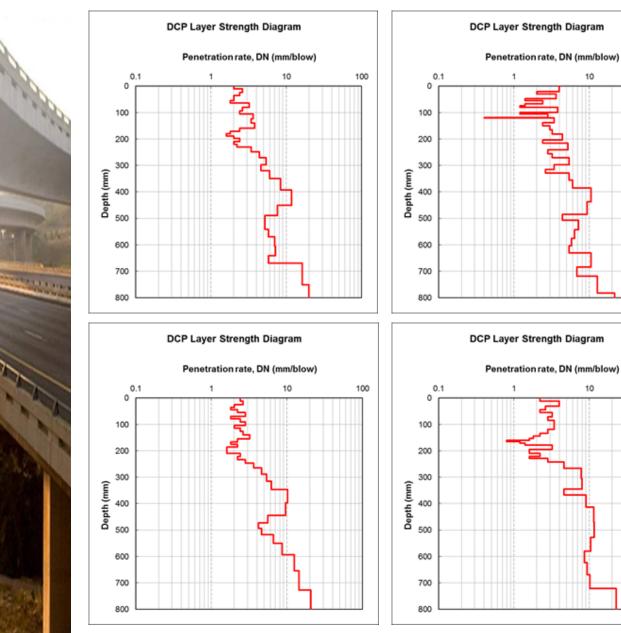


# **ADDS AGEN VY 200 FTB2 km 22 NB – Deflection**





#### **EXAMPLE A MATCHAN 200 FTB2 km 22 NB – DCP**





100

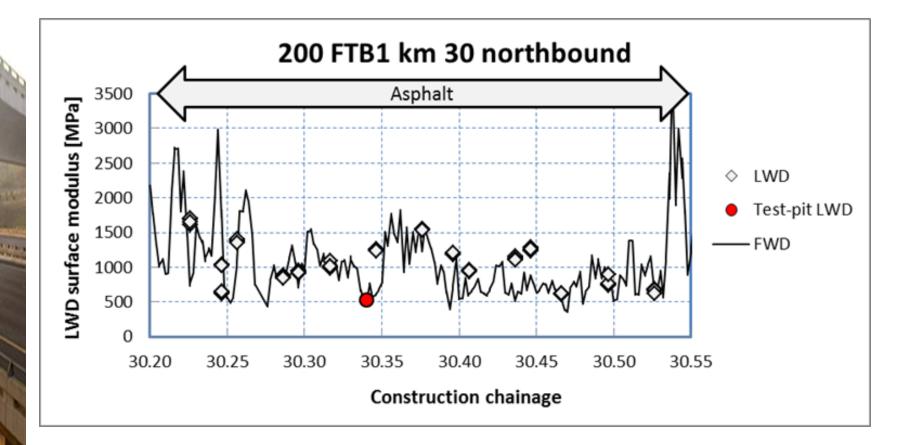
100

# <sup>1</sup> 200 FTB2 km 30 – Test-pit visual

ROADS AGENCY

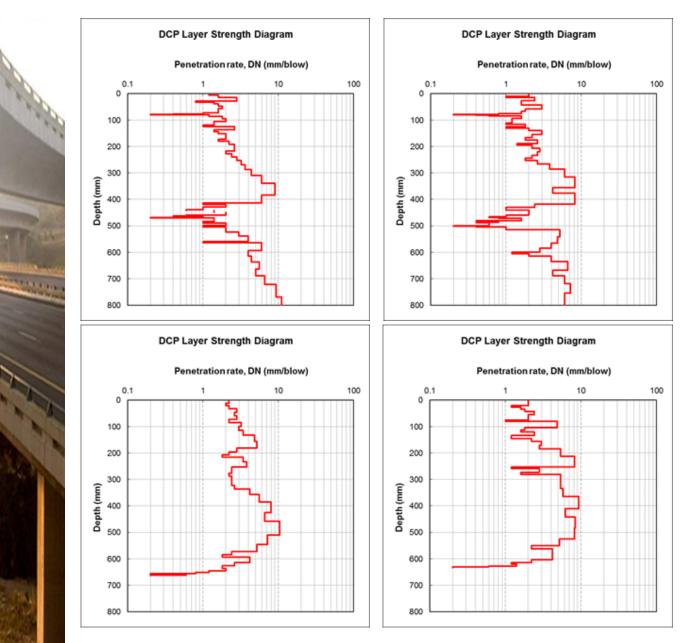


# **200 FTB2 km 30 NB – Deflection**



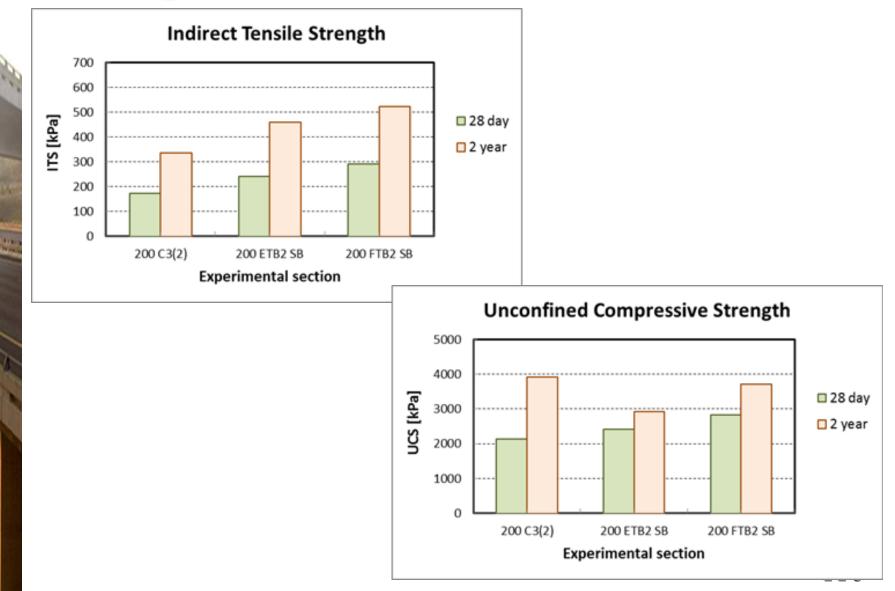


#### **EXAMPLEAN MARKAN 200 FTB2 km 30 NB – DCP**

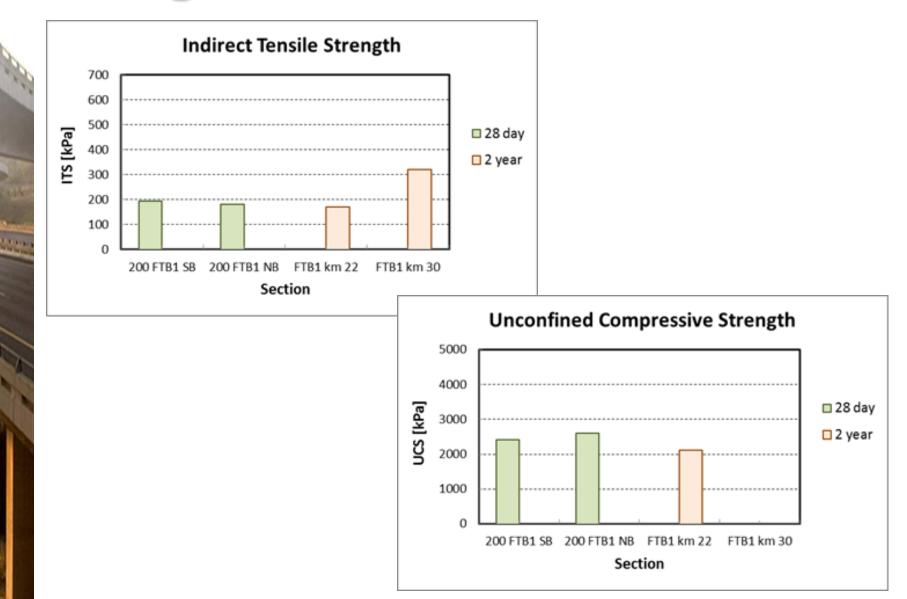


 $P_m^c$ 

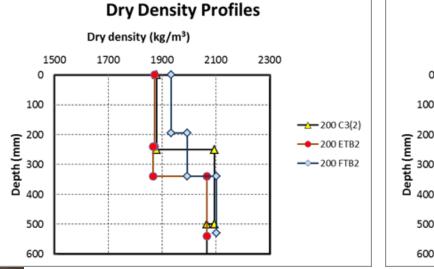
### Combined results – Material strength – 2 % cement

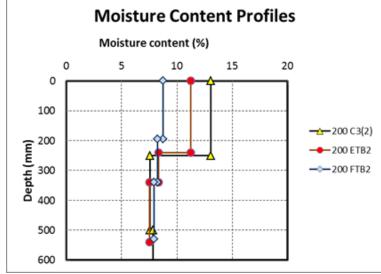


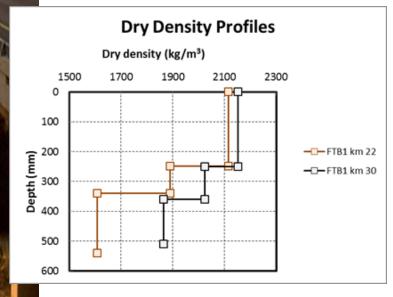
### Combined results – Material strength – 1 % cement

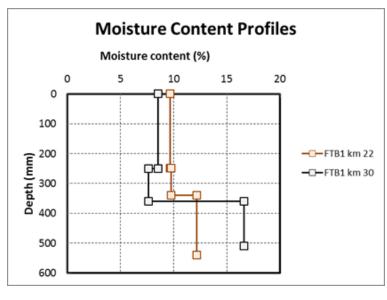


### **Combined results – Density and moisture content**





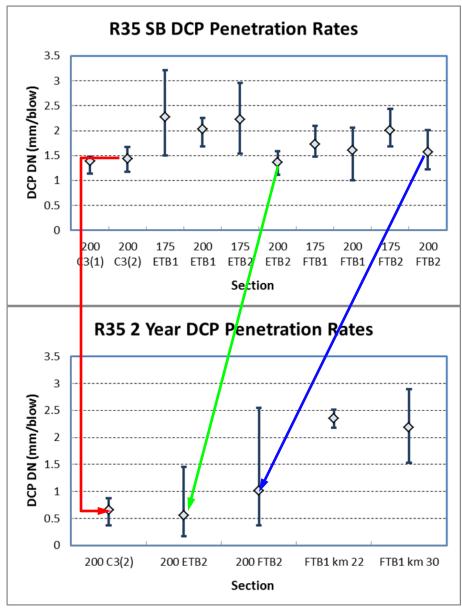




P<sub>m</sub><sup>c</sup>

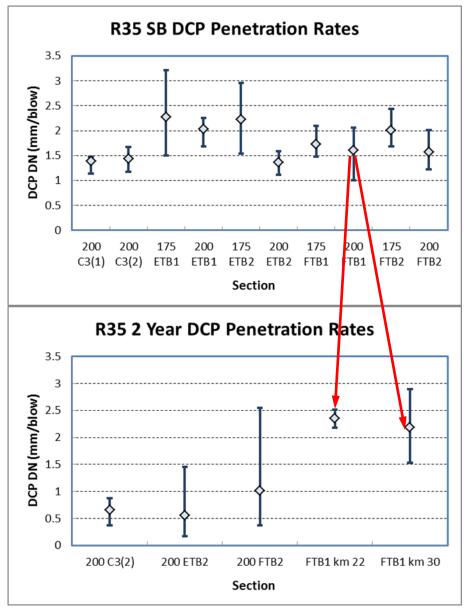
# **Combined results – DCP**

P. P.





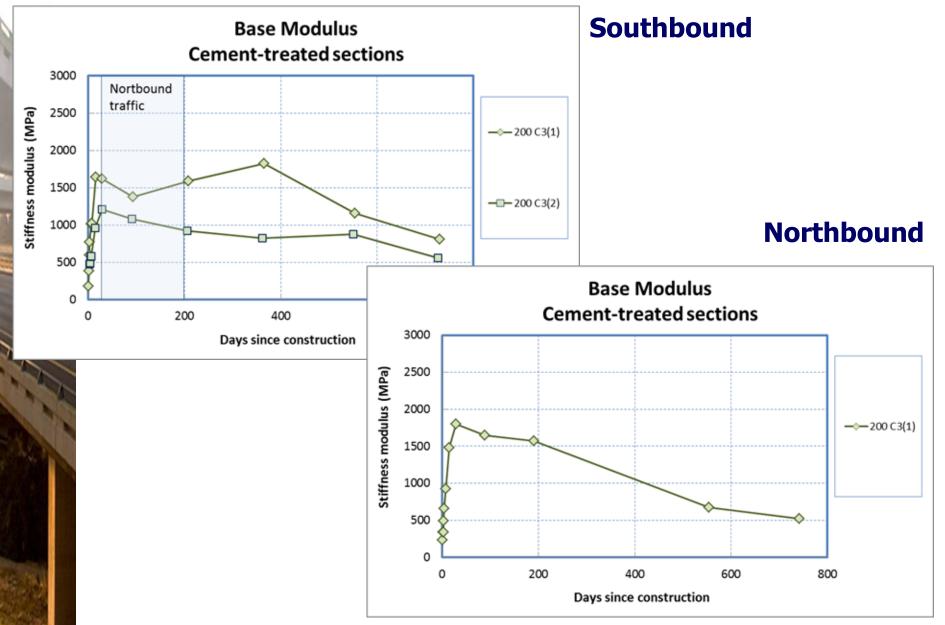
# Combined results – DCP



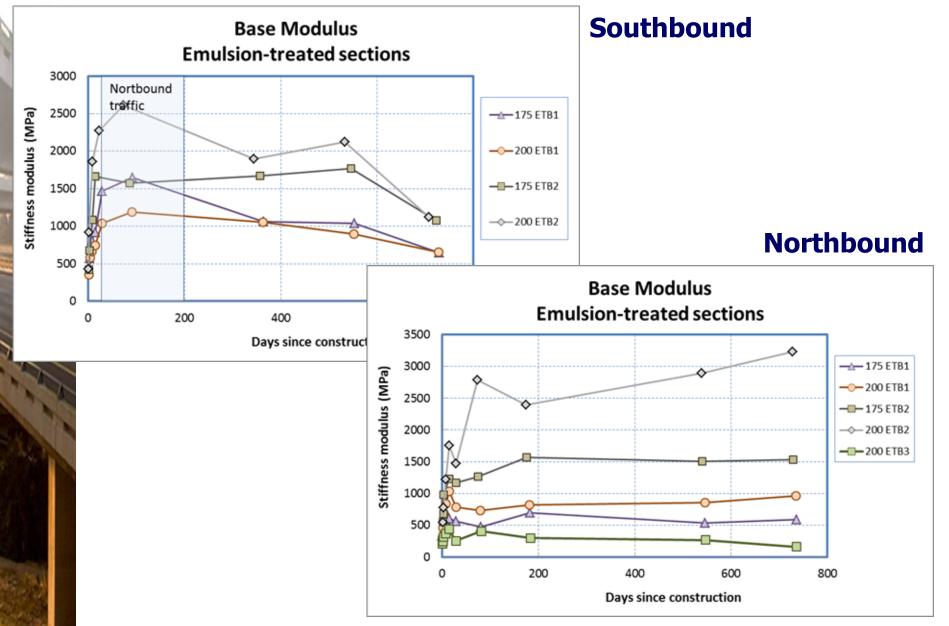


# Combined results – FWD

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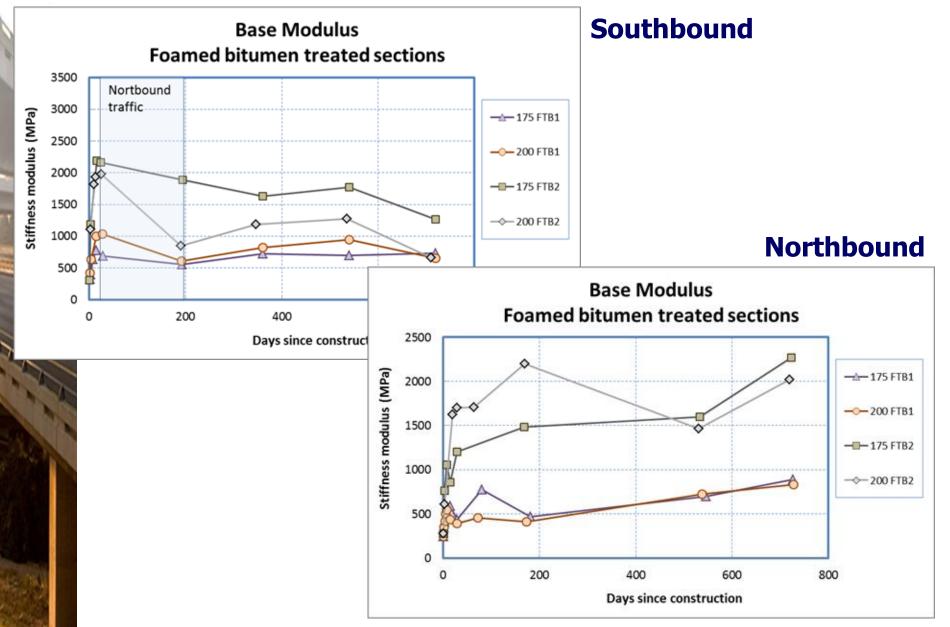


# Combined results – FWD



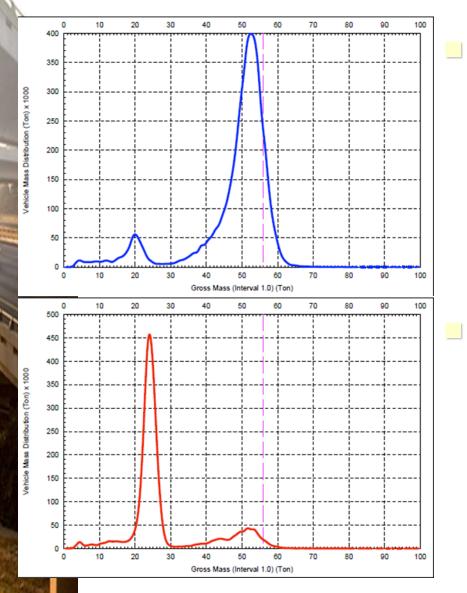
## **Combined results – FWD**

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### **Combined results – Traffic**



#### Southbound

- 155 947 HV in 2013
- 706 196 E80 in 2013
- 997 552 E80 from
  Aug 2012 to April 2014

#### Northbound - **152 916** HV in 2013 - 134 301 E80 in 2013

– 179 068 E80 from Jan
 2013 to April 2014



## Comparison with design

#### Design traffic estimate

- -1.2 meSA for 2 years
- -1.0 meSA from Aug 2012 to April 2014

### **Recorded traffic**

- -Southbound
  - 0.99 meSA from Aug 2012 to April 2014
- -Southbound
  - 0.18 meSA from Jan 2013 to April 2014



### **Comparison with design – Cemented crushing**

- Design estimate of area affected by crushing after 1 meSA
  - Asphalt surfacing 10 %
  - Cape seal 30 %
- Outcome



 Very difficult to determine extent but some crushing may have been observed at core locations on cape seal sections





#### **Comparison with design – Stiffness reduction**

- Design estimate of area to reach constant stiffness after 1 meSA
- Outcome
  - No rigorous analysis done yet
  - Difference between cement and BSM shown by both design models and field observation
  - BSM emulsion retained higher field stiffness than
    BSM foam not shown by design

Ceme	nt	BSM f	oam			BSM emulsion					
C3	C3 FTB1			FTB2		ETB1		ETB2			
AC	S4	AC	S4	AC	S4	AC	S4	AC	S4		
30 %	48 %	0%	1%	0	1%	1%	2%	0%	1%		



### **Comparison with design – Permanent deformation**

Design estimate of average rut after 1 meSA

#### Outcome

- Detailed rut measurements still to be done
- Difference between Cape seal and asphalt not identified at design stage
- Design estimates not far removed from field

Shear strength parameters used in design not the usual published values

	Cement		BSM fo	am			BSM emulsion					
	C3		FTB1		FTB2		ETB1		ETB2			
	AC	S4	AC	S4	AC	S4	AC	S4	AC	S4		
Design	0.2	0.2	0.2	0.3	0.1	0.1	1.1	1.1	0.5	0.5		
Field	0.8	5.3			0.3	5.5			1.7	3.8		



## Conclusions

- Very little distress on experimental sections after 2 years
  - Deep, well-balanced pavements
- Problems on mainlines sections related to subgrade problems
  - Identification during design stage?

Past design models are not fundamentally flawed but input must be correct

