

Modify and Expand the Concrete
Pavement Design Method, cncPAVE
to Include Block Pavement Design
and to add it into the SAPDM

TASKS

The further development and refinement of cncPAVE including the following:

1. Load transfer model for cracks and joints to include the effect of steel fibre reinforcement
2. Riding quality deterioration models based on IRI
3. Interaction between the concrete slab and the subbase with special reference to erosion of this layer
4. Incorporate a mechanistic-empirical design method for block paving into cncPAVE
5. Integration of cncPAVE into the SAPDM system.

1. Load transfer model for cracks and joints

The extent to which a load can be transferred from one slab to the adjoining slab depends on:

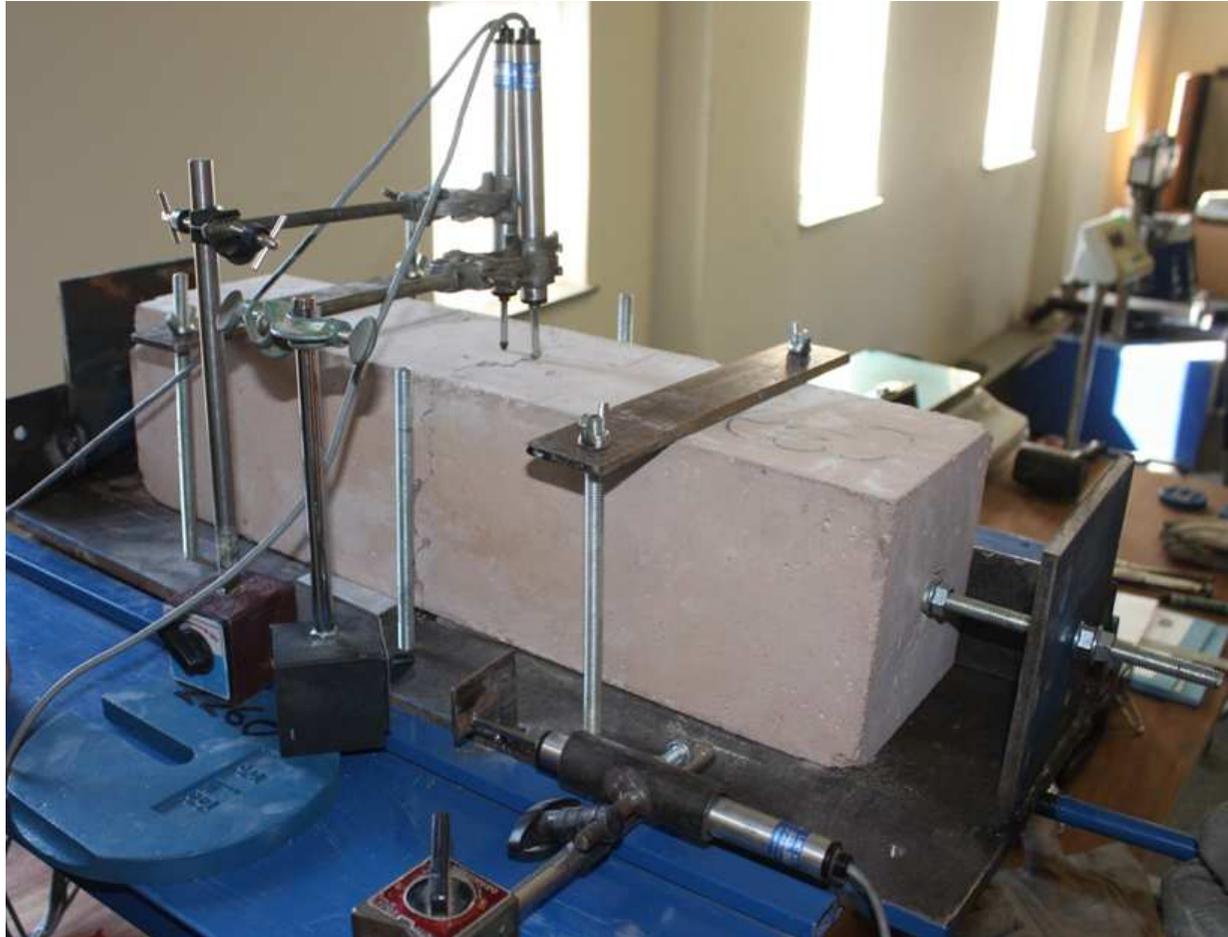
- The crack/joint width, thus shrinkage, crack/joint spacing etc.
- Non-uniformity in the joint/crack thus concrete mix properties, aggregate size, ACV
- Reinforcement/dowels/steel fibres that may traverse the crack/joint
- The magnitude and number of traffic loads at a joint/crack

Quantify the magnitude of load transferred at a crack or joint

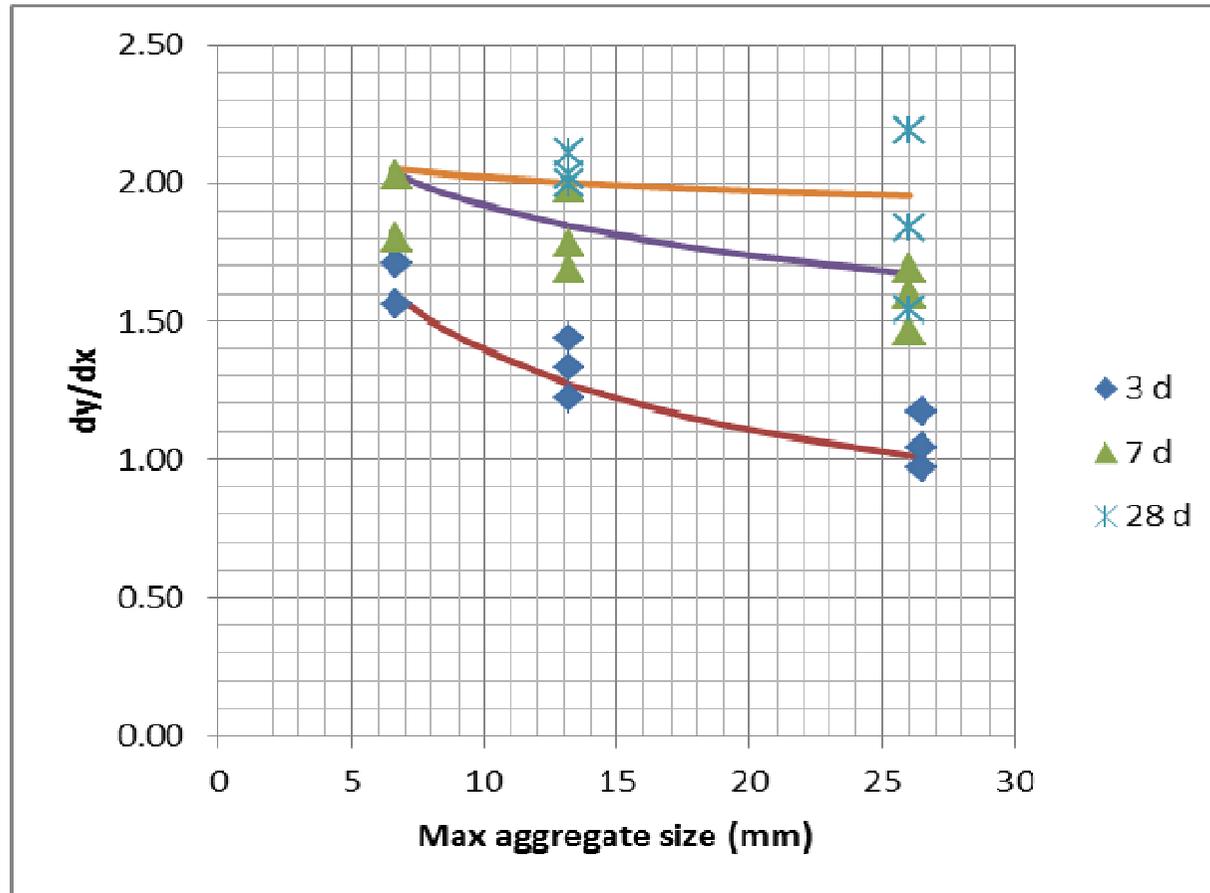
Manufacture beams using different concrete mixes and fibre contents. Test the following:

- First set of beams: Measure flexural strength at different ages
- Second set of beams: Just initiate a crack and then vary the crack width and measure the movement across the crack at different loads

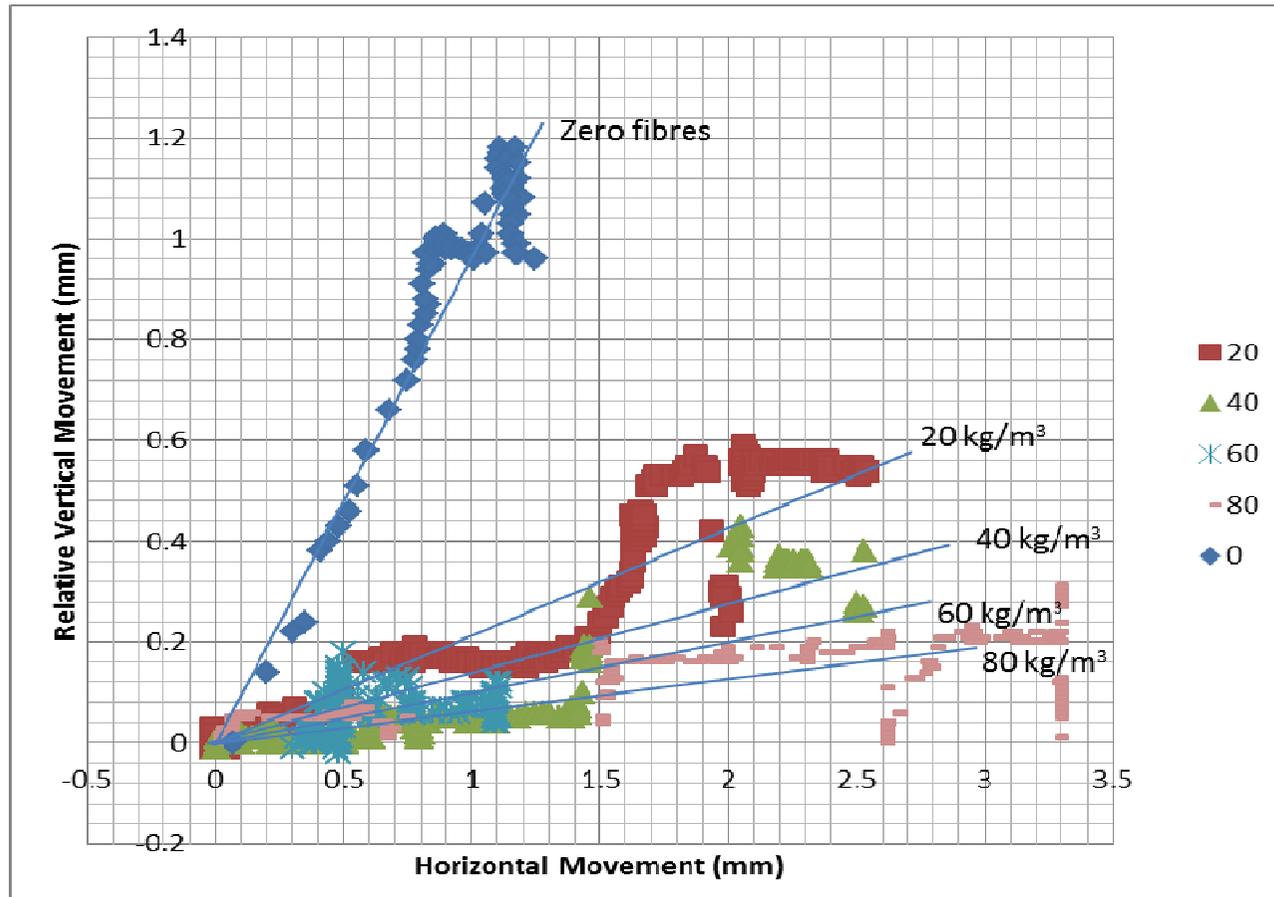
Laboratory Testing



Results so far: Aggregate size



Results so far: Fibre Content



2. Pavement Distress and International Roughness Index (IRI)

IRI measured using instruments and used to determine road users cost and pavement performance. It therefore depends on:

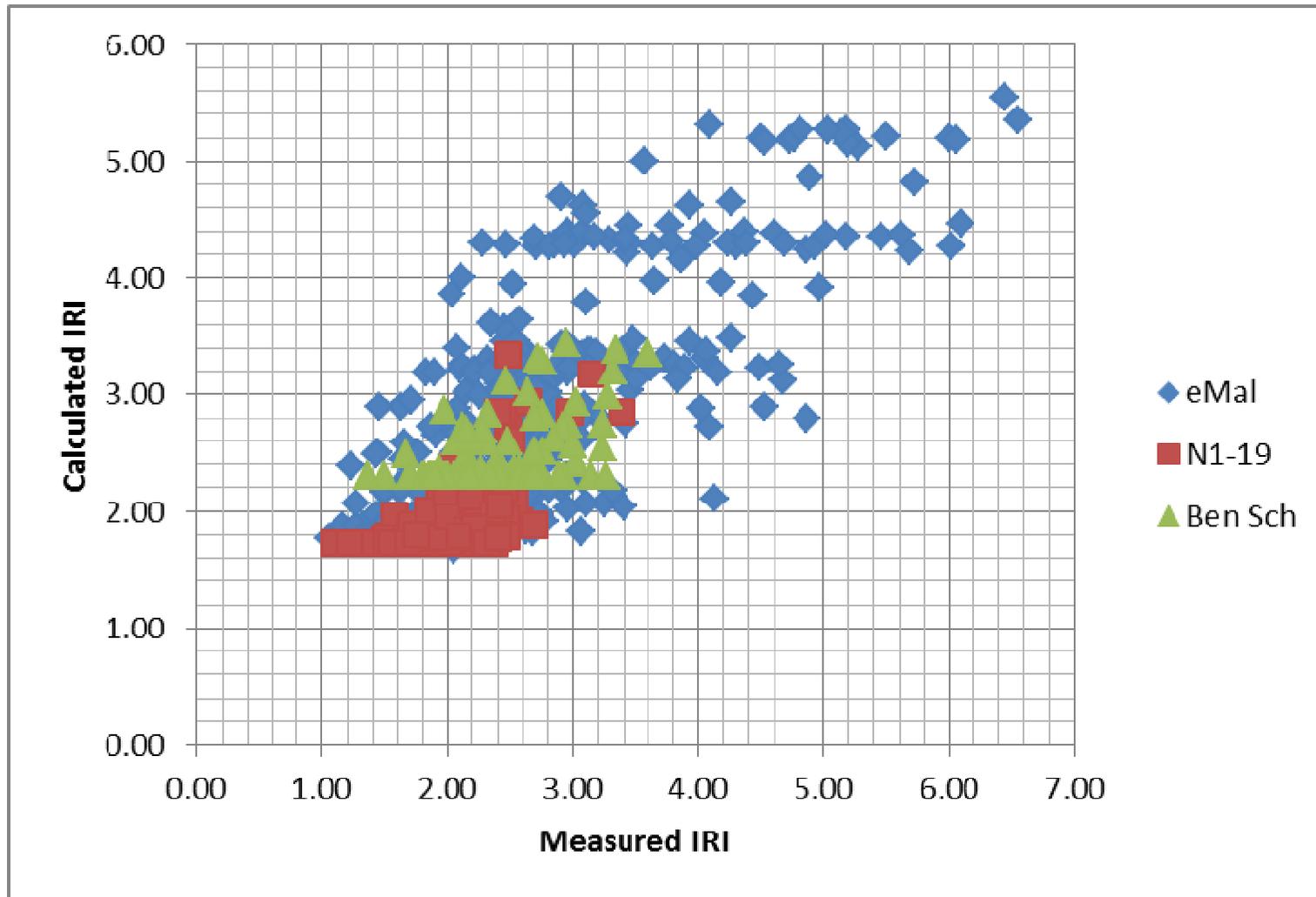
- Initial roughness after construction
- Shattered slabs
- Faulting at joins/cracks
- Repairs

Evaluation of IRI versus Distress

Many concrete sections have been rehabilitated after distress but the following was very useful:

- N1/19 : JCP south of Johannesburg
(Vaal River to Misgund)
- N4/3: JCP east of eMalahleni
(Witbank to Middelburg)
- N1/19 & 20: CRCP Pretoria to Johannesburg
(Ben Schoeman highway)

Linear plot of Adjusted Data



Prediction

IRI = f(Initial IRI, Area shattered,
area repaired, faulting at joints)

3. Erosion of the subbase

- The performance of a concrete slab depends to a great extent on slab support
- High deflection, subbase with low stiffness and water cause erosion (pumping)
- Brush test being used to quantify erodibility
- A factor (constant) is presently being used
- Investigate use of the Rotational Shear Device to enable a mechanistic approach



Torque Transducer

Stabilising Arm

Thrust Bearing

Top Disk

Sample

Rotating Cylinder

Bottom Disk

Test Method Considered at this Time

- Sample, 100mm diameter and 100mm high prepared using standard lab procedures
- Rotate at 1500 rpm for between 0.5 and 2 hours
- Material's loss to be between 0 and 10% depending on resistance to erosion
- Hopefully stripping of bituminous layer below slab can also be determined

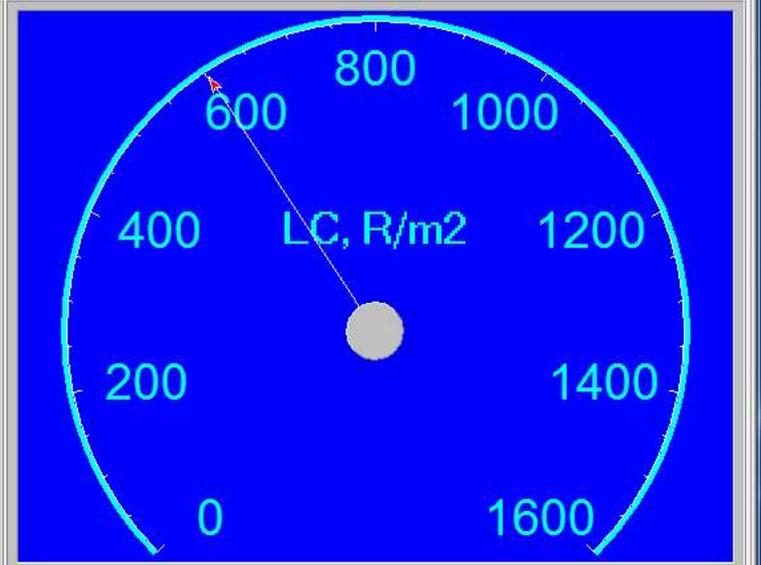
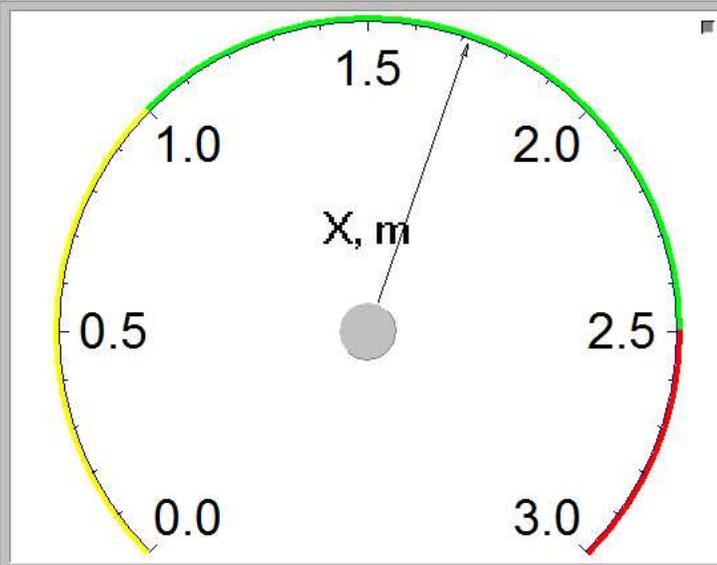
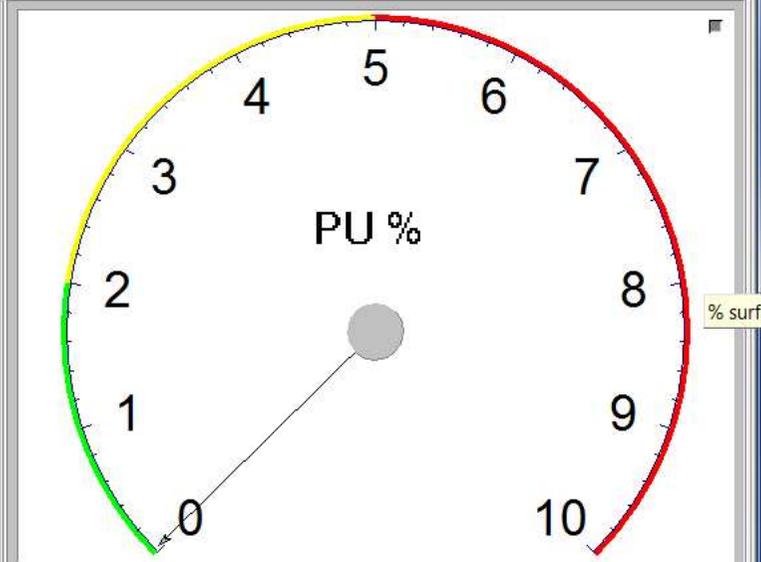
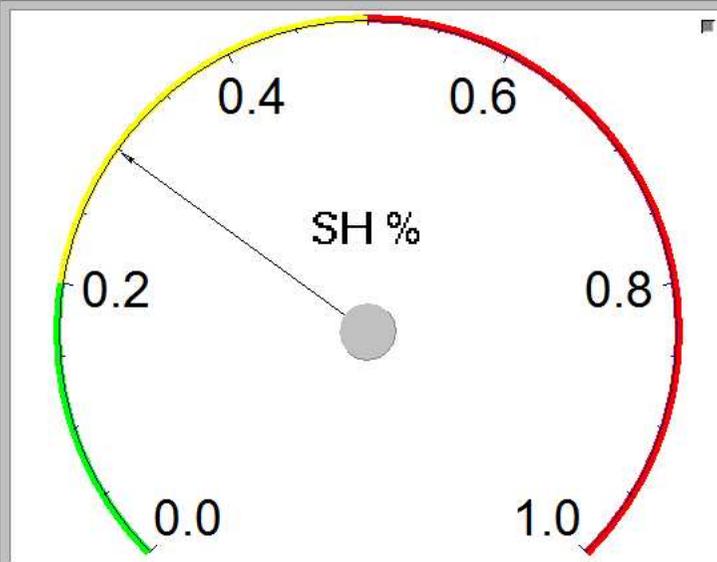
4. Adding the Design of Interlocking Block Pavements

Use the same page layout as being used in cncPAVE:

- Control page for input of variables and constants
- Traffic load page with typical distributions and the capability to use real data obtained
- Facts page showing the detailed outcome of a run
- Pages showing typical distribution and changes with time of the more important output
- What-if to plot the outcome as a function of different input parameters.

V	Variable	Min q	Best	Max q
1	Cement content, kg/m3	0.9	340	1.10
2	Water content, l/m3	0.80	170	1.50
3	Flexural strength, MPa	0.75	4.2	1.60
4	Layer 1 (slab) h1, mm	0.95	200	1.10
5	Layer 2 (subb) E2, MPa	0.67	1200	1.50
6	Layer 2 (subb) h2, mm	0.85	125	1.20
7	Layer3 E3, MPa	0.80	150	1.30
8	Layer3 h3, mm	0.80	150	1.30
9	Layer4 E4, MPa	0.80	120	1.30
10	Layer4 h4, mm	0.80	150	1.30
11	Layer5 E5, MPa	0.60	80	2.00
12	Growth of HV traff., %pa	0.85	3.5	1.15
13	Speed of HV, ave km/h	0.20	60	2.00
14	Contact pressure, MPa	0.50	0.60	2.00
15	Annual rainfall, mm p.a.	0.70	760	1.30
16	Daily temp. cycle, C	0.60	18	1.50
17	Joint movement, mm	0.10	0.0	2.50
18	Void extra Vex, m	0.01	0.0	2.00

C	Constant	Value
1	Life period, yrs	40
2	ADTT0, initial HV/day/lane	1400
3	Axles/HV, average	4.543
4	Loads on edge, %	0
5	Damage constant a	1200
6	Damage exponent b	4.5
7	Ratio k1 = E1/f	8000
8	Ratio k2 = ITS/f	0.77
9	Bond	8
10	Drainage factor	3
11	Erosion factor	4
12	Joint spacing, m	0
13	Steel diameter, mm	16
14	Steel spacing, mm	170
15	Aggregate Crushing Value	20
16	Aggregate type	1
17	Aggregate size, mm	37
18	Cement type	1
19	Aggreg. content factor	0.72
20	Fibre content, kg/m3	0.0



Keep input: CR UC U

Type of concrete pavement:

Description of site, time and run:

Simulation: 1000

Import Export

Abort RUN

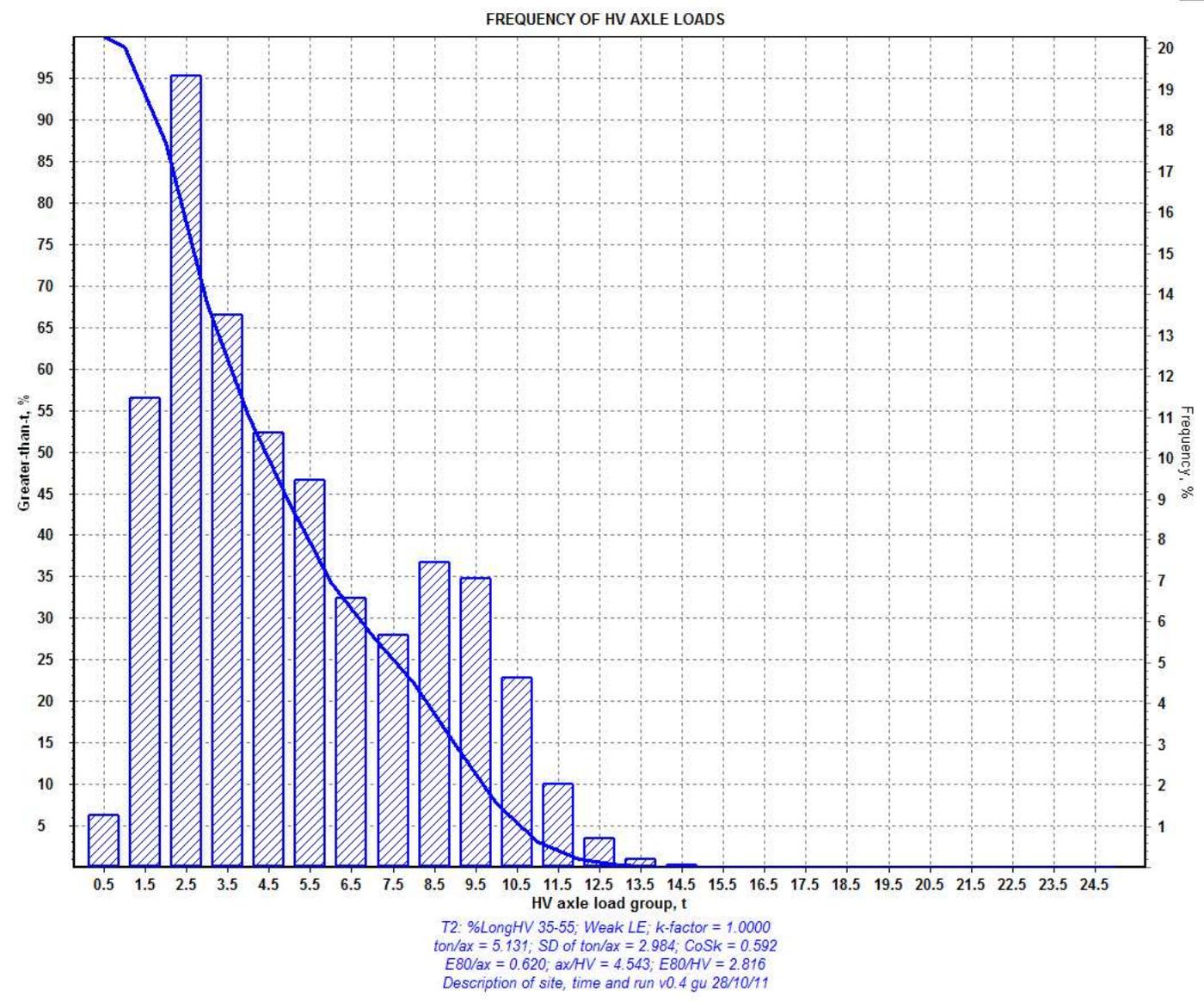
1000 simulations done in 0.03 s

Time t1 to 1st rehab. yrs: 20.0

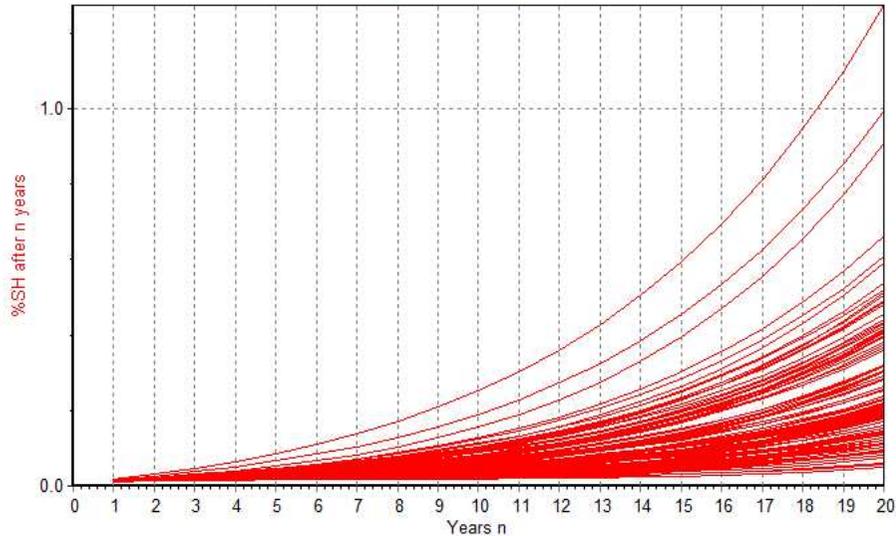


TRAFFIC SITUATION

	Load, t	Freq. %	k-factor
<input type="radio"/> T1: %LongHV < 35; (Any LE*)	0.5000	1.27	1.000
<input checked="" type="radio"/> T2: %LongHV 35-55; Weak LE	1.5000	11.46	
	2.5000	19.32	
<input type="radio"/> T3: %LongHV 35-55; Strong LE	3.5000	13.50	
	4.5000	10.63	
<input type="radio"/> T4: %LongHV > 55; Weak LE	5.5000	9.48	
	6.5000	6.56	
<input type="radio"/> T5: %LongHV > 55; Strong LE	7.5000	5.66	
	8.5000	7.45	
<input type="radio"/> Streets in heavy-industry areas	9.5000	7.05	
	10.5000	4.61	
<input type="radio"/> Residential streets	11.5000	2.04	
	12.5000	0.70	
<input type="radio"/> Heavy-truck standing areas	13.5000	0.21	
	14.5000	0.06	
<input type="radio"/> Light-truck standing areas	15.5000	0.00	
	16.5000	0.00	
<input type="radio"/> One single axle load	17.5000	0.00	
<input type="radio"/> Other traffic situation			

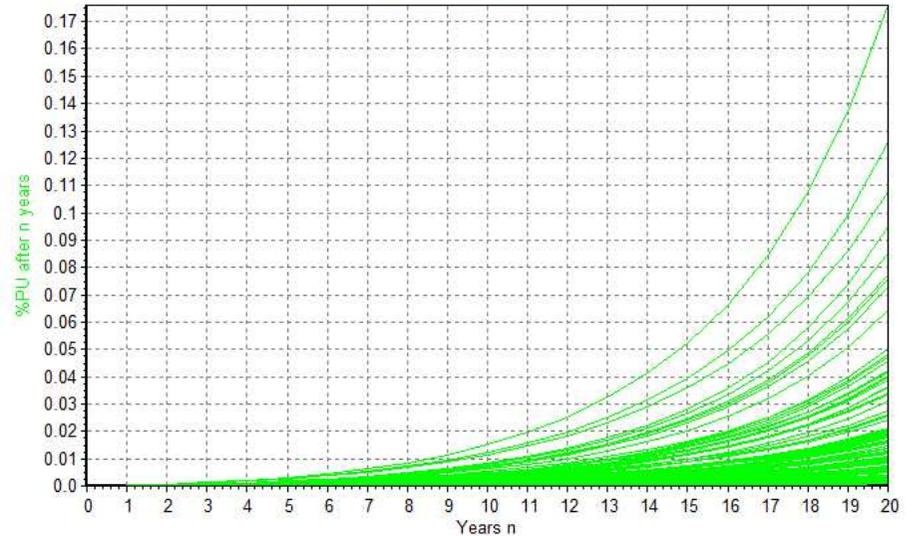


SURFACE WITH SHATTERED CONCRETE



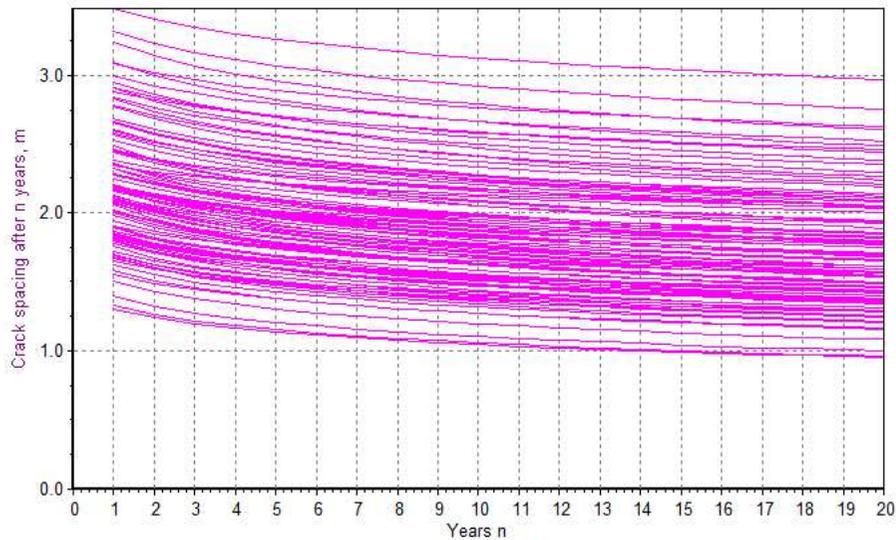
Description of site, time and run v0.4 gu 28/10/11

PERCENTAGE OF SURFACE AREA PUMPING



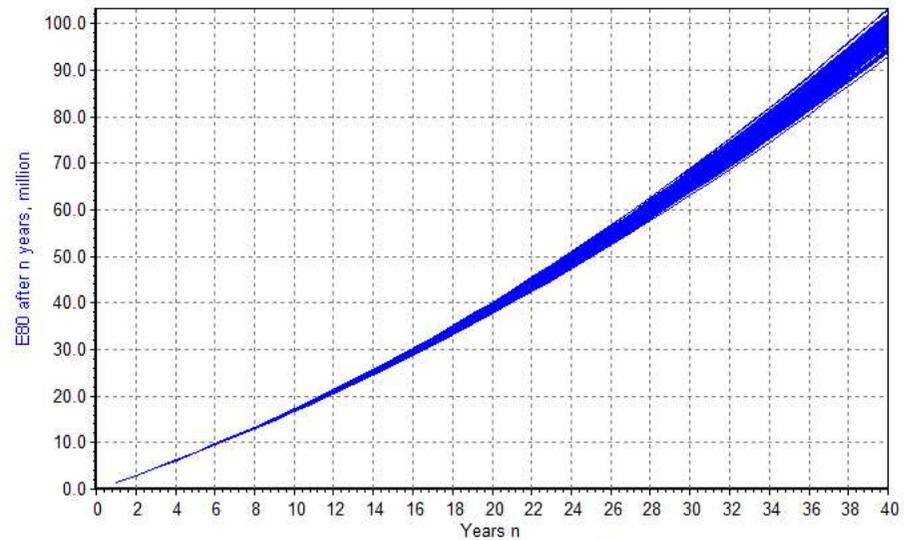
Description of site, time and run v0.4 gu 28/10/11

CRACK SPACING DUE TO SHRINKAGE



Description of site, time and run v0.4 gu 28/10/11

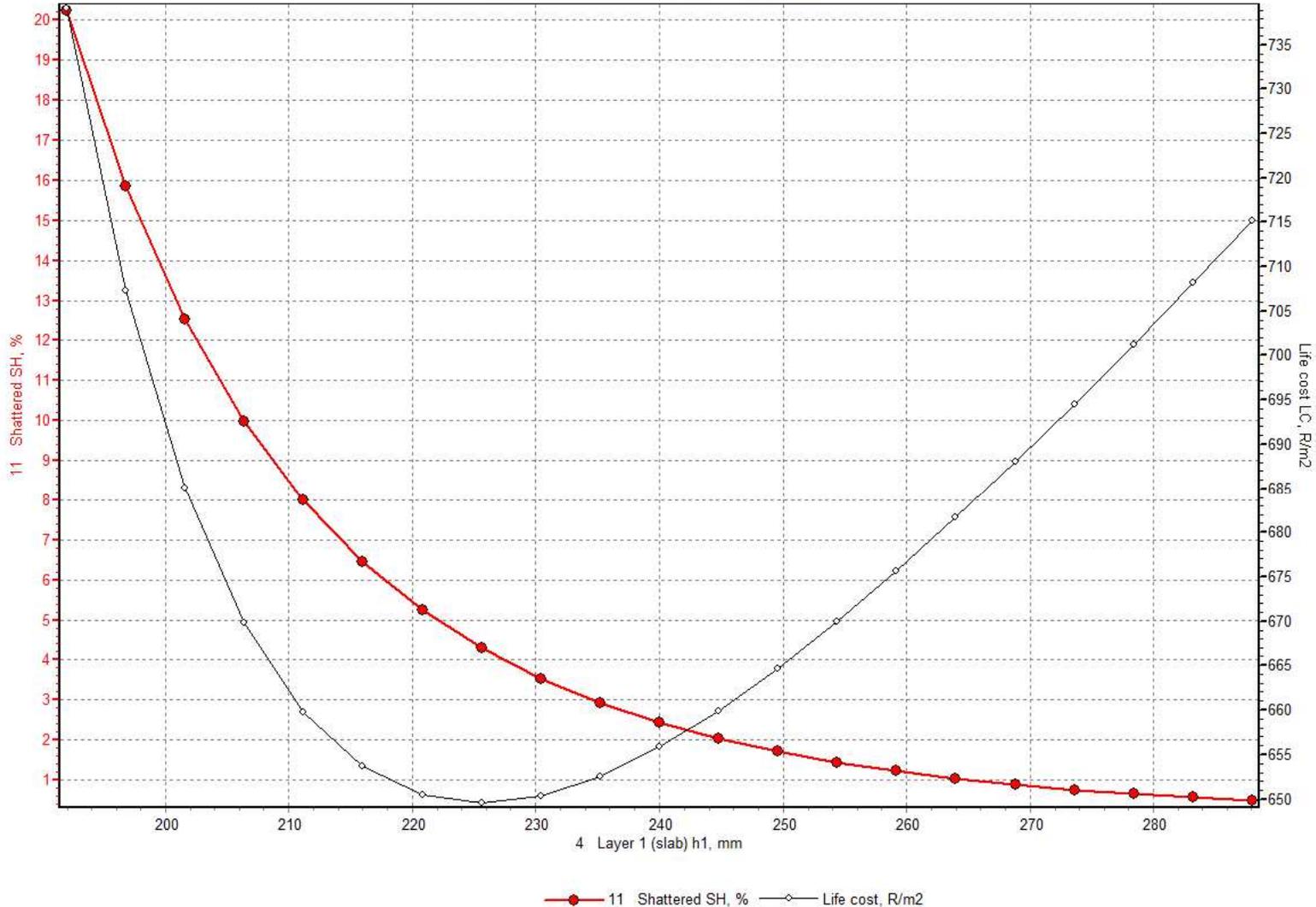
BUILD-UP OF E80 OVER LIFE PERIOD



Description of site, time and run v0.4 gu 28/10/11

? Help

WHAT-IF EXPERIMENT: TRACK OF AVERAGES



Description of site, time and run v0.4 gu 28/10/11

Print...

Click an input item below:

No	Input to test
1	Cement content, kg/m3
2	Water content, l/m3
3	Flexural strength, MPa
4	Layer 1 (slab) h1, mm
5	Layer 2 (subb) E2, MPa
6	Layer 2 (subb) h2, mm
7	Layer3 E3, MPa
8	Layer3 h3, mm
9	Layer4 E4, MPa
10	Layer4 h4, mm
11	Layer5 E5, MPa
12	Growth of HV traff., %p
13	Speed of HV, ave km/h
14	Contact pressure, MPa
15	Annual rainfall, mm p.a.
16	Daily temp. cycle, C

Click an output item below:

No	Output to watch
1	Ca
2	Steel Cs
3	Stiffness Ee, MPa
4	Stress Sts, MPa
5	HV axles n, million
6	Total E80, million
7	Curl, m
8	Deflection, mm
9	Crack spac. X, m
10	Void, m
11	Shattered SH, %
12	Pumping PU, %
13	Faulting FA, %
14	Constr. cost, R/m2
15	PW rehab., R/m2
16	Life cost, R/m2

Point: 21 Input value: 240

21-POINT RANGE WIDTH: Zero ±10% ±20%

VARIABILITY: No Yes



Development

Block pavement divided into two components:

- The paving blocks with sand filled joints placed on top of a bedding sand about 30mm thick.

Finite element analysis the best tool.

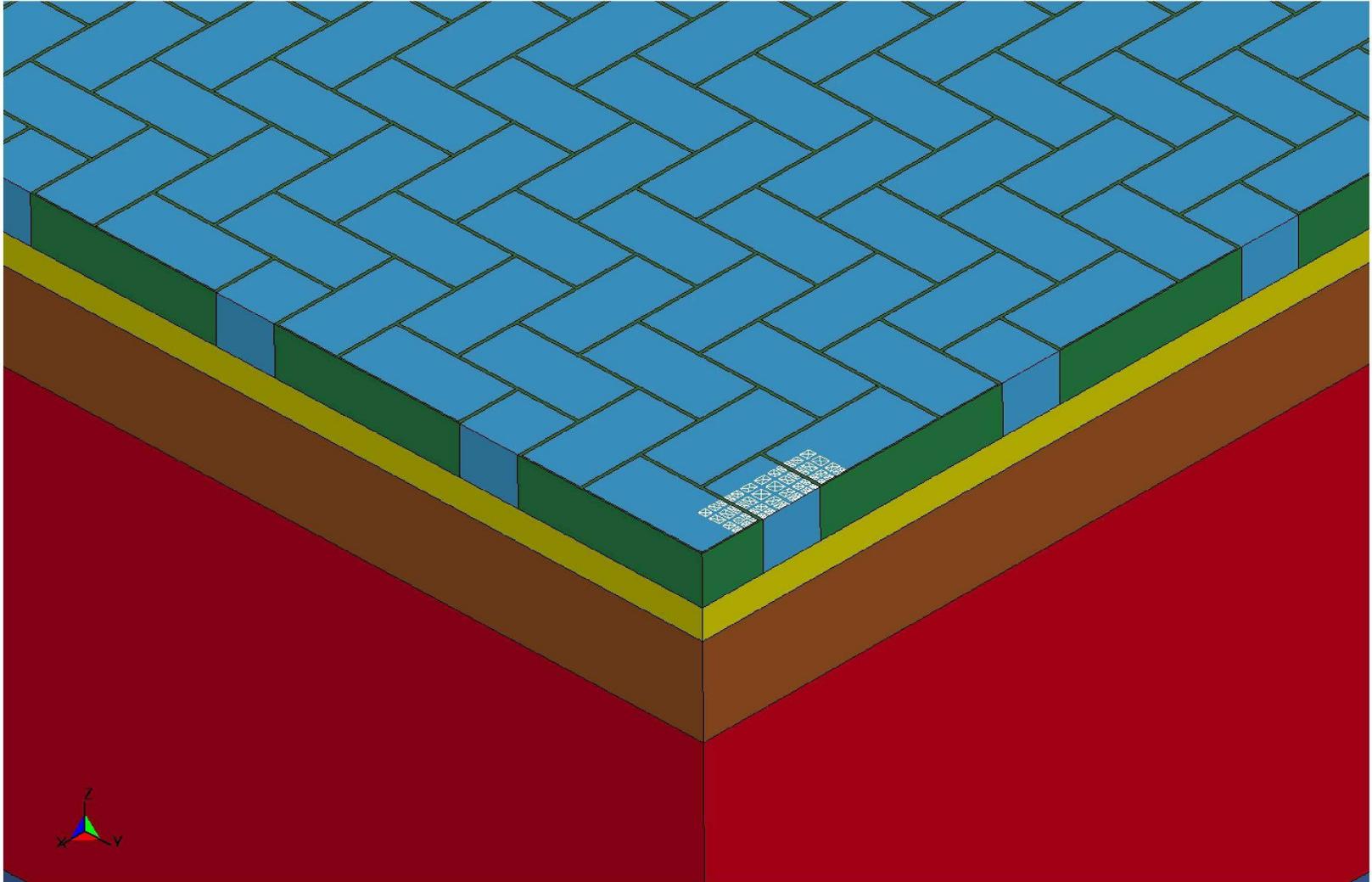
- The subbase and lower layers below the bedding sand. Normal multi-layer analyses can be used in this case. Ties in with approach used in flexible pavement design.







3-D Finite element model

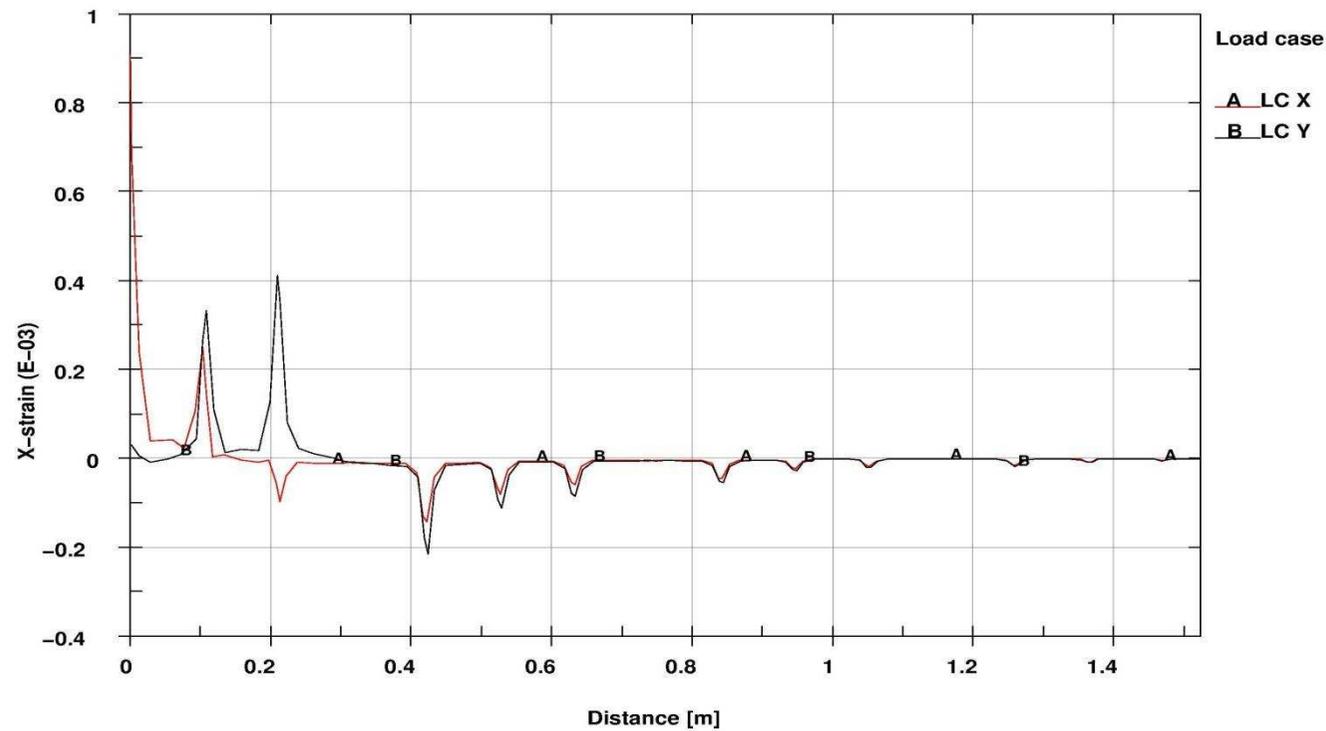


Blocks, Joints and Bedding Sand

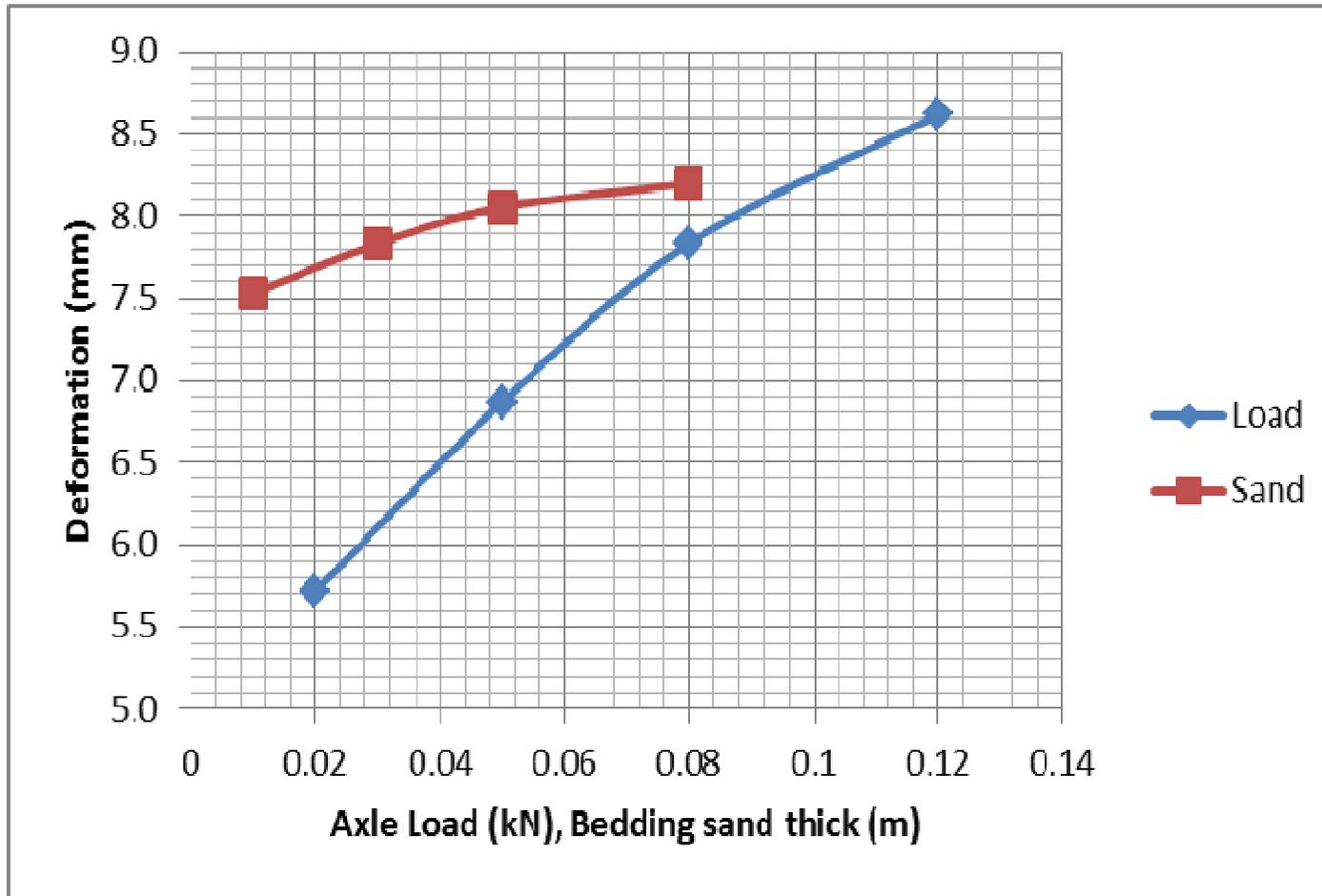
Finite element analysis shows:

- Tensile strain at the bottom of the joint between blocks and at the top of the bedding sand
- Bedding sand tend to move away from joint between blocks
- Blocks tend to move apart and joints between blocks tend to open
- Water entering aggravates the situation
- Result: Deformation of the pavement, voids below blocks resulting in loose blocks that may also crack

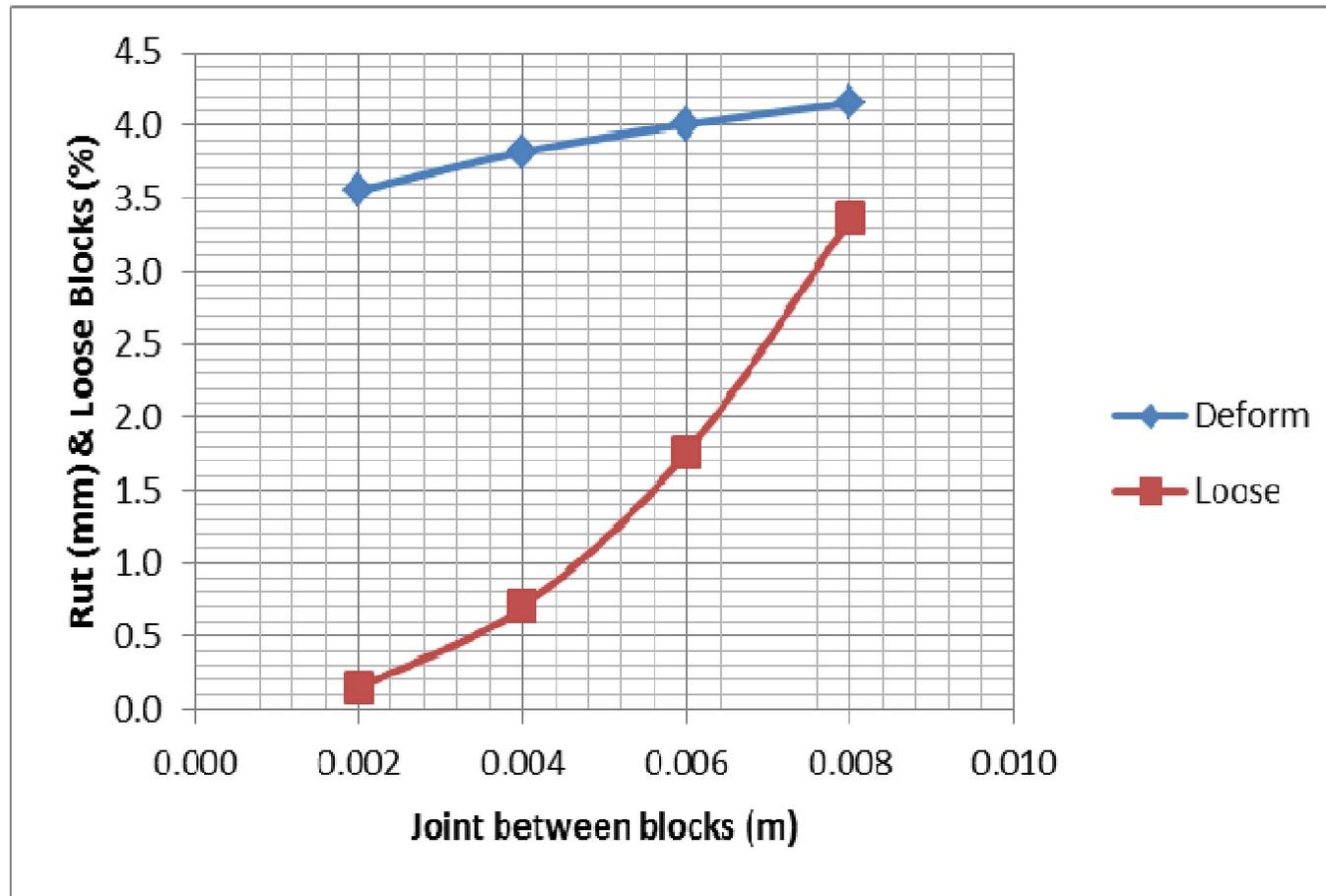
Strain below the blocks at the surface of the bedding sand



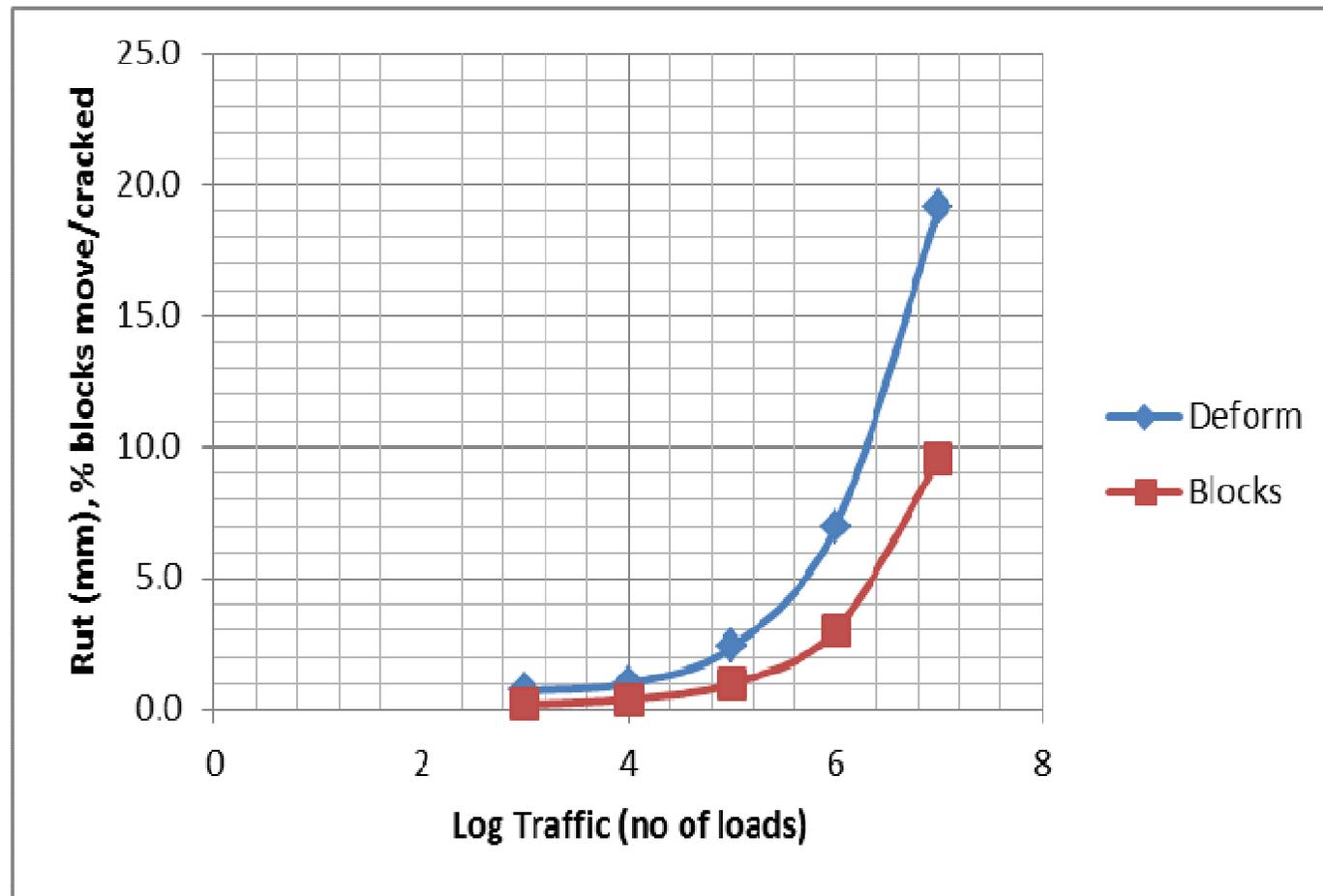
Effect of axle loads & bedding sand thickness on deformation



Effect of joint width on deformation & loose/cracked blocks



Deformation and cracked/moving blocks with increased number of loads



Way Forward

- Adjust the load transfer model in cncPAVE
- Refine the models for predicting IRI
- Develop test method to quantify erosion
- Develop the computer program to design block pavements
- Refine all models using field data and practical experience and integrate it into cncPAVE
- Incorporate cncPAVE into the SAPDM.

Assistance from industry to calibrate the
block pavement design method by
providing data on performance will be
greatly appreciated.

Thank you

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