Mechanistic-Empirical Structural Design Models for Emulsified Bitumen Treated Materials

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**Sponsored by: SABITA and GAUTRANS** 







#### Background

#### ◆ TG2 for foamed bitumen treated materials

- Published in September 2002
- Uses mechanistic-empirical structural design
- Emulsified bitumen treated materials
  - SABITA Manuals:
    - » Manual 14: GEMS (1993) and 21:ETB (1999)
  - Manual 21: ETB, no mechanistic-empirical design models
    - » Catalogues based on DCP design method, validated by empirical catalogues
  - Lots of field experience

# Terminology

#### Previous differentiation between

- Modification: 0.6 1.5% residual binder
- Stabilisation: 1.5 5 % residual binder
- Artificial

» based on what is put in, not on behaviour achieved

- Move away from differentiation between modification and stabilisation
  - Performance based material classification
  - Refer to material as
    - » "emulsified bitumen treated material" (EBTM)
  - One design method for all EBTMs

# **Structural design models:** What is required?

#### ♦ HVS

- Pavement behaviour and pavement performance data

» Identify distress mechanisms

#### – Data

» Resilient moduli (initial and reduction)

» Permanent deformation



#### ♦ Laboratory

- Engineering, mechanical and durability data
- Preferably for same material as HVS tests

*Material classification and structural design* 



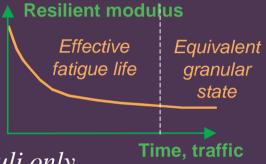


### **HVS Tests**

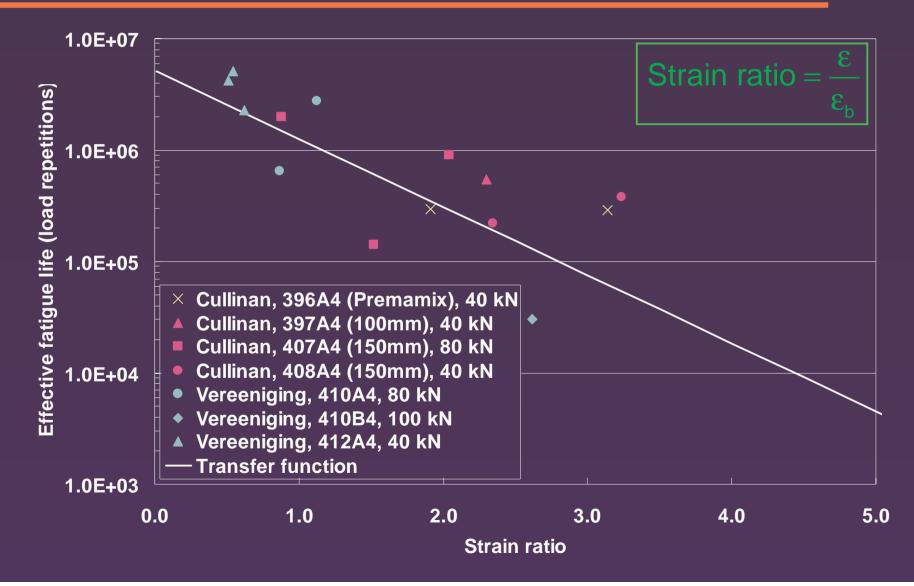
HVS Tests	Heilbron (P9/3)	Cullinan (D2388)	Vereeniging (P243/1)							
Date	1992	2000	2001							
Construction	Conventional	Labour	DISR							
Material	weathered dolerite	sandstone congolomerate	ferricrete							
Material class	G4/G5	G5	G7							
Residual binder	0.6, 1.2, 1.8%	0.9%	1.8%							
Filler	2.5% lime*	1% cement	2% cement							
HVS sections	5	4	2							
Lab tests										
Beam	×	<b>J</b>	<b>J</b>							
Triaxial	×		1							
UCS & ITS	×	<b>J</b>	<b>√</b>							
*lime pretreatment										

## **Appropriate transfer functions**

- Selected from observations of HVS tests
  Effective fatigue
  - Initial resilient moduli reduce under load
  - Reduce to an "equivalent granular state"
    - » Equivalent to granular material in resilient moduli only
    - » Not necessarily in loose, particulate state
  - Related to flexibility
- Permanent deformation
  - Permanent deformation accumulates during test
  - Long time period than effective fatigue
    - » Therefore Phase 2 of pavement life
  - Related to shear strength



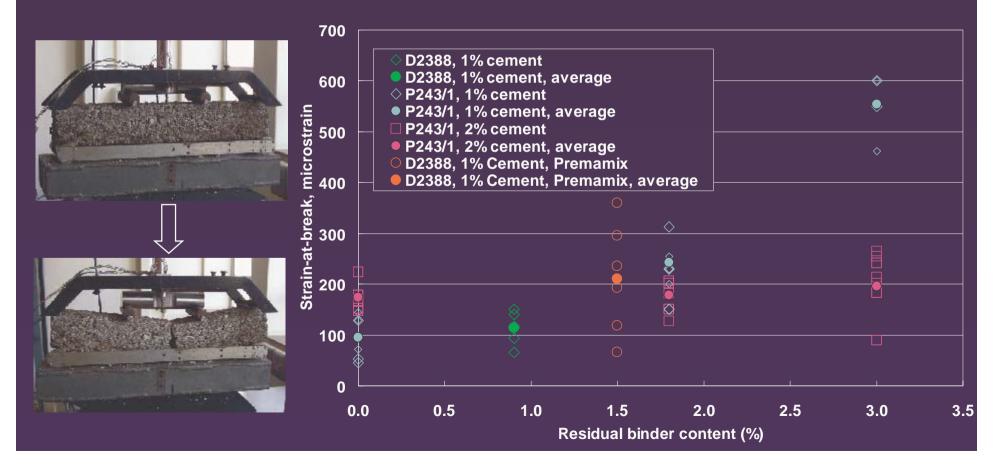
#### **Effective Fatigue**



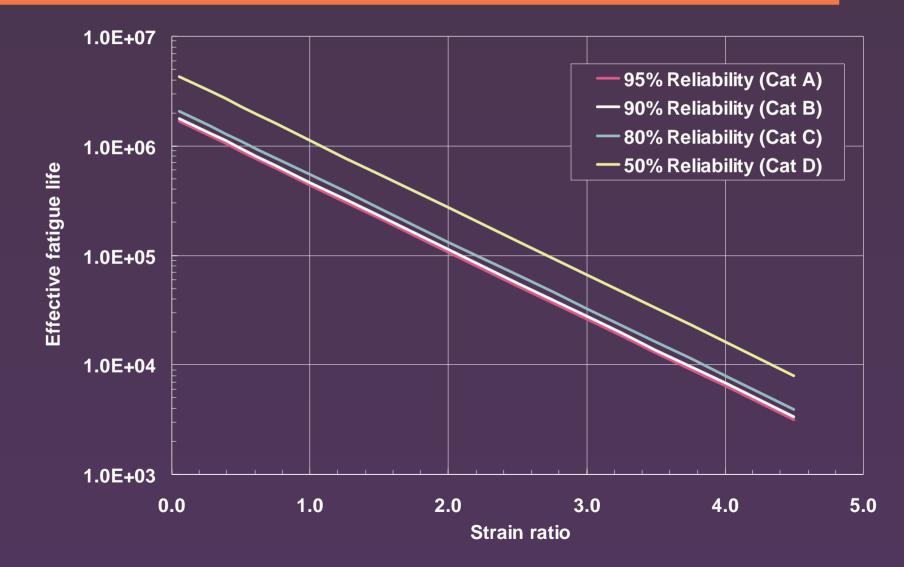
#### Flexibility

# Increases with increasing binder content

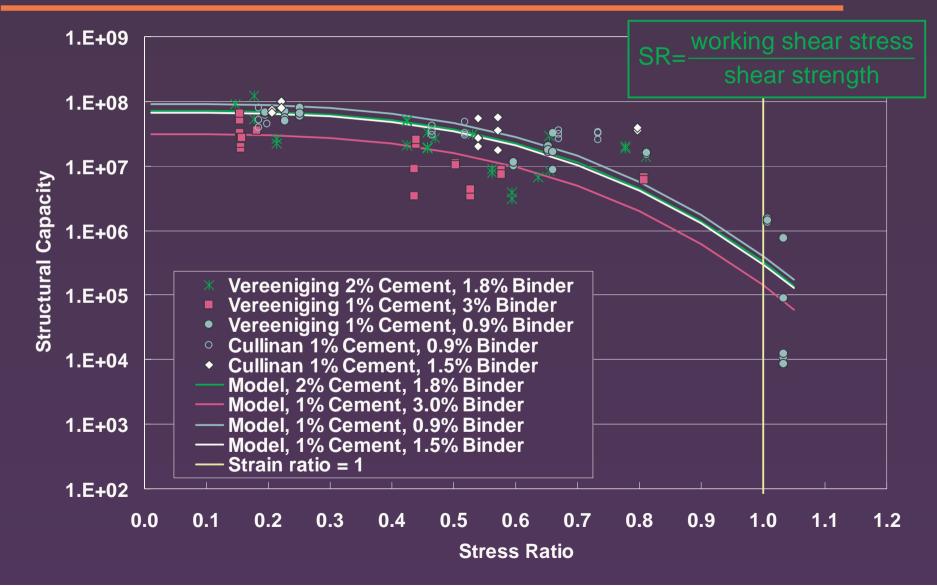
#### – If cement content not too high



#### **Effective Fatigue**

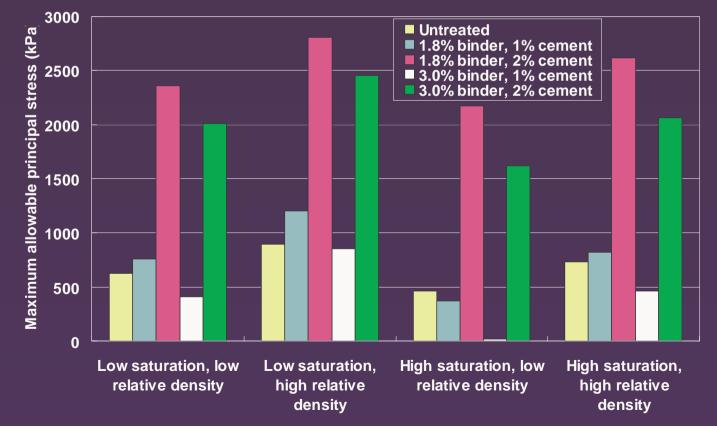


#### **Permanent deformation model: Lab**

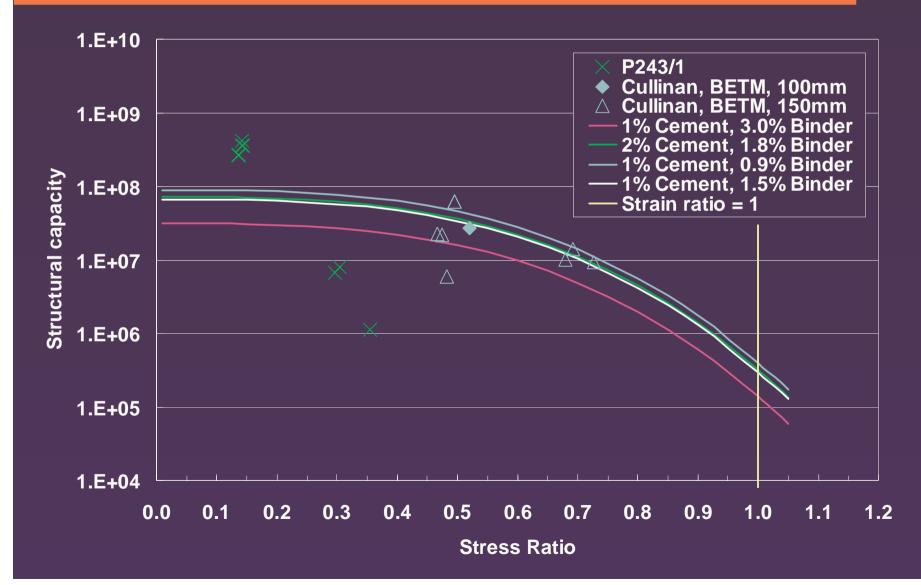


## **Shear strength (resistance to PD)**

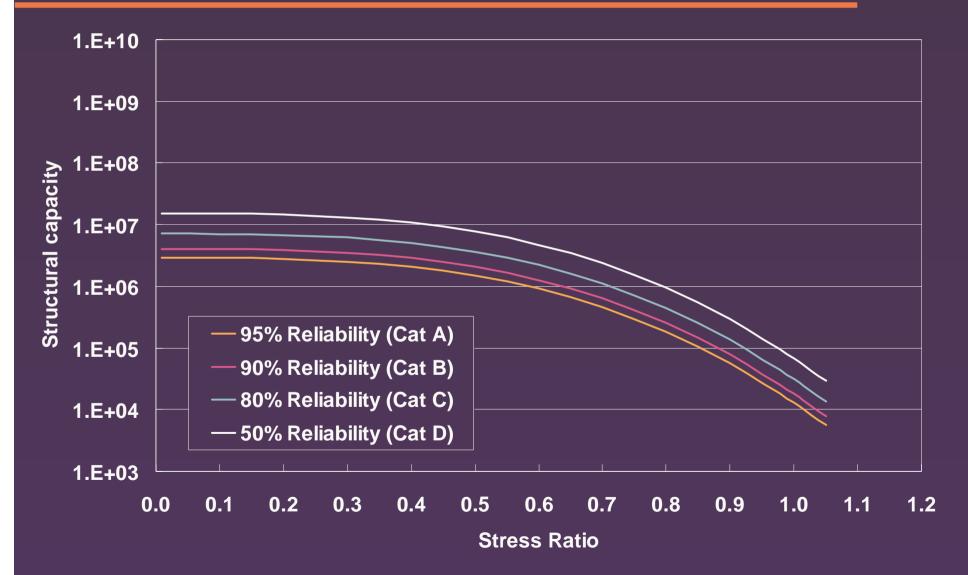
- Increase shear strength
  - Increase in relative density
- Decrease shear strength
  - Increase in saturation
- Increase in cement content
- Increase in binder content



# Permanent deformation model: Lab with HVS data



# Permanent deformation transfer function



#### **Material classification**

Must capture structural behaviour

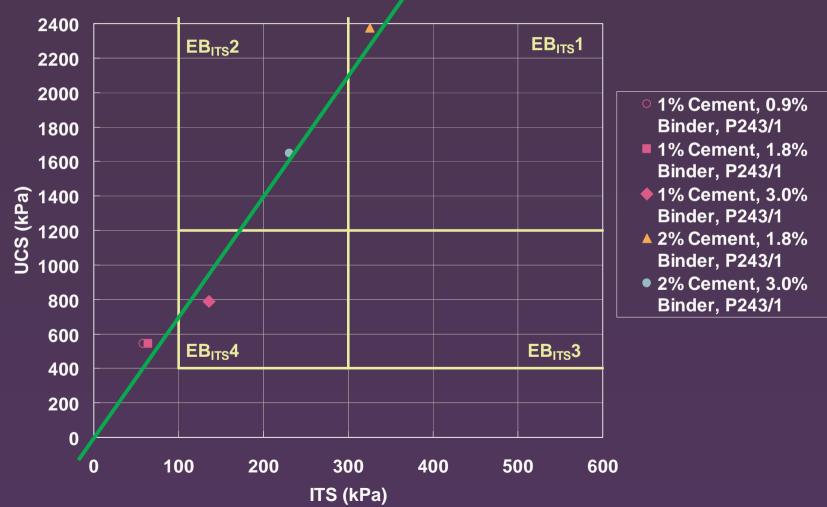
 Flexibility
 Resistance to permanent deformation

 Investigating UCS, ITS and ε<sub>b</sub>

 UCS and ε<sub>b</sub> seem to work well for EBTMs to date
 Needs further refinement

#### **Material classification**

#### ♦ UCS and ITS



#### Material classification

#### • UCS and strain-at-break



# Catalogue: New construction

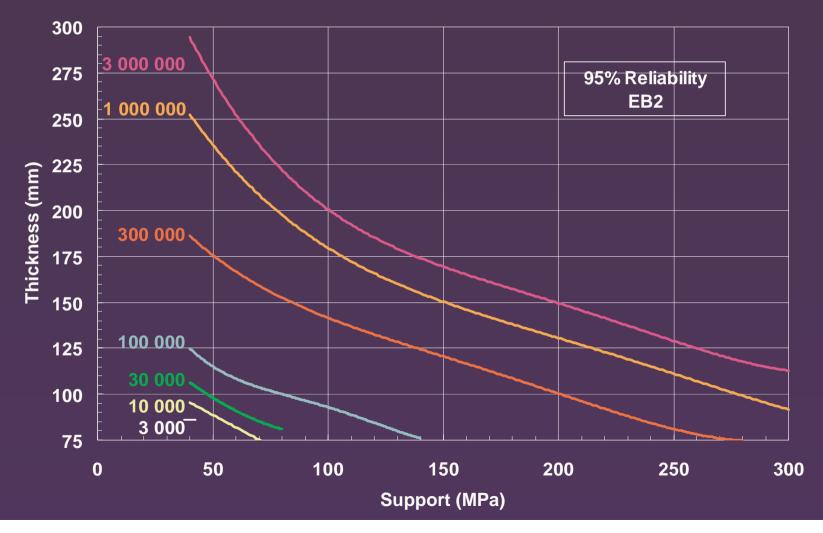
NEW CONSTRUCTION: EMULSIFIED BITUMEN TREATED BASE (EB2)											
	PAVEMENT CLASS AND DESIGN BEARING CAPACITY (80 kN AXLES/LANE)										
ROAD CATEGORY	ES0,003 0,1-0,3x10 <sup>4</sup>	ES0,01 0,3-1,0x10 <sup>4</sup>	ES0,03 1,0-3,0x10 <sup>4</sup>	ES0,1 3,0-10x10 <sup>4</sup>	ES0,3 0,1-0,3x10 <sup>6</sup>	ES1 0,3-1,0x10 <sup>6</sup>	ES3 1,0-3,0x10 <sup>6</sup>	ES10 3,0-10x10 <sup>6</sup>	ES30 10-30x10 <sup>6</sup>	ES100 30-100x10 <sup>6</sup>	Foundation
A: Major interurban freeways and roads. (95 % approximate design reliability)						SEAL 150 EB2 150 C4 SEAL 200 EB2 200 G6	30AC 150 EB2 200 C4* 30 AC 200 EB2 200G6				
<ul> <li>B: Interurban collectors and major rural roads.</li> <li>(90 % approximate design reliability)</li> </ul>					SEAL 100B2 100 C4 SEAL 125 EB2 125 G6	SEAL 125 EB2 150 C4 SEAL 175 EB2 175 G6	30 AC 150 EB2 175 C4 30 AC 200 EB2 200 G6				150 G7 150 G9 G10
C: Lightly trafficked rural roads and strategic roads. (80 % approximate design reliability)				SEAL 100 EB2 100 G6	SEAL 100B2 100 C4 SEAL 100 EB2 125 G6	SEAL 125EB2 125 C4 SEAL 175 EB2 175 G6	30 AC 125 EB2 175 C4 30 AC 175 EB2 175 G6				
D: Light pavement structures, rural access roads. (50 % approximate design reliability)				SEAL 100 EB2 100 G8							150 G9 G10

SYMBOL A DENOTES AG, AC, OR AS. SYMBOL S DENOTES S2 OR S4 A0, AP may be recommended as a surfacing measure for improved skid resistance when wet or to reduce water spray.

\* Minimum thickness for structural capacity. Construct in two lifts increasing thickness for ease of construction, if necessary.

## Design Charts: Rehab design

#### Design charts (EB2, 95% reliability)



# **Conclusions and recommendations**

- Now have ME structural design models
  - Effective fatigue
    - » Dependant on flexibility
  - Permanent deformation
    - » Density and saturation, additives often more important than load
- Material classification
  - UCS and strain-at-break
- Design catalogues and charts
  - Pavement designs similar to those recommended by consultants, and developed by DCP design method
- Foamed and emulsified bitumen
  - Essentially same structural capacity

### What's next?

◆ ETB laboratory testing (EB1?) Strain-at-break test Compactability study - Foamed and emulsified bitumen ♦ Field validation ◆ Guideline document, TG3 ? All HVS and associated laboratory testing reports www.gautrans-hvs.co.za Questions?

